

Hastings District Council

Whakatu Arterial Link Project Description

June 2014

Table of contents

1.	Introd	duction	1
	1.1	Purpose and Scope of this Document	1
	1.2	The Whakatu Arterial Project	1
	1.3	Abbreviations	2
	1.4	Structure of this Document	3
2.	Back	ground Information	3
	2.1	HPTS	3
	2.2	Enquiry by Design (EBD) Process	4
3.	Loca	tion	7
	3.1	Project Location and Description of Route	7
	3.2	Affected properties	8
4.	Road	I Design	9
	4.1	Investigations informing design	9
	4.2	Pavement Design	11
	4.3	Alignment detail	11
	4.4	SH2, Pilcher Road and Napier Road Intersection	13
	4.5	Pakowhai Road Intersection	14
	4.6	Local Road Intersections	15
	4.7	Farndon Road Intersection	16
	4.8	Rail Level Crossing Design	17
	4.9	Closure of the Ruahapia Road Level Crossing	17
	4.10	Cycleways	18
	4.11	Karamu Stream Bridge Design	18
	4.12	Stormwater Design	21
	4.13	Street Lighting Design	22
5.	Proje	ct Construction	23
	5.1	Construction Environmental Management Plan	23
	5.2	Project Construction Schedule	23
	5.3	Construction overview	24
	5.4	Construction workforce	26
	5.5	Hours of construction work	26
	5.6	Property access	26
	5.7	Temporary facilities	27
	5.8	National Environmental Standards on Air Quality	27
	5.9	Earthworks	27
	5.10	Potential impacts from construction	27
6.	Oper	ation and Maintenance	30
	6.1	Operation	30

	6.2	Maintenance	30
	6.3	Potential impacts from operation and maintenance	31
7.	Basis	of Report	37

Table index

Table 1: Summary of Total Land Requirements	8
Table 2 Design Traffic Assumptions – Whakatu Arterial	10
Table 3– Traffic Multipliers used in CIRCLY Pavement Design Software	11
Table 4: Street Lighting Design Summary	23
Table 5 Accident Cost Summary	32
Table 6 Do Minimum and Project 2026 Annual Daily Traffic Flows	32
Table 7 Waahi Tapu Sites	35

Figure index

Figure 1: Route options identified in the HPTS	4
Figure 2 Route Alignment	6
Figure 3: Indicative route location	7
Figure 4: Typical cross section	12
Figure 5: Indicative layout of SH2 / Whakatu Arterial / Napier Rd Roundabout	13
Figure 6: Indicative layout of Pakowhai Rd / Whakatu Arterial Roundabout	14
Figure 7 Whakatu Road Intersection	15
Figure 8: Farndon Road Intersection Improvements - Preferred Option 4 Layout	17
Figure 9 Karamu Stream Cross Section with Bridge	19
Figure 10 Cross Section of Karamu Stream Bridge	20
Figure 11 Minimum Swale Cross Section	22
Figure 12 Location of Waahi Tapu Sites (1)	36
Figure 13 Location of Waahi Tapu Sites (2)	36

Appendices

Appendix A - Technical Drawings Appendix B – Lighting Subcategory Evaluation Appendix C – HBRC Spill Management Plan

1. Introduction

1.1 Purpose and Scope of this Document

The Project Description Report (PDR) has been prepared to describe the proposed Whakatu Arterial Link (WAL) between SH2 North and Pakowhai Road near Hastings. It outlines the approach taken to select a preferred road alignment, the investigations conducted to inform key design decisions, and details of road design and location including key design elements such as major intersections and the crossing of the Karamu Stream.

This PDR will be used as the basis for applications under the Resource Management Act 1991 (RMA) to designate the proposed route and other related resource consents.

This document does not assess the impacts of construction or operation of the proposed road on the environment. It does however describe and quantify the project such that others may assess these impacts. To that end, separate environmental effects studies have been undertaken by others using the parameters set out in this document.

A Stormwater Management Plan (GHD 2014g) and Erosion and Sediment Control Plan (GHD 2014h) and a draft Construction Environmental Management Plan (GHD 2014i) have been developed and are included with the RMA application documentation. At such time as the WAL proceeds to the construction phase, these plans will be included in tender documentation together with copies of conditions imposed under the RMA.

1.2 The Whakatu Arterial Project

The WAL has been identified in the Hawke's Bay Regional Land Transport Strategy 2012 - 2042 short term programme.

The WAL provides a strategic link between SH2 North and Pakowhai Road with a view to achieving the overall objectives of the WAL, which are to:

"...enhance and improve the safety and efficiency of the transport network of the district and region so as to:

- Improve accessibility for individuals and businesses and support economic growth and productivity;
- Provide convenient, efficient and safe access for freight movements to and from the Whakatu Industrial Area;
- Promote the use of the Hawkes Bay Expressway for the road transport of freight and vehicles between the Whakatu Industrial Area and the Port of Napier;
- Provide convenient, efficient and safe access between Havelock North and the Napier/Hastings Airport and Napier's north western employment and residential areas; and
- Enhance the safety of the Whakatu residential area by reducing freight movements through it."

1.3 Abbreviations

The abbreviations used in this report are listed in the following Table:

Abbreviation	Description
AADT	Annual Average Daily Traffic
AC	Asphaltic Concrete
AEE	Assessment of Environmental Effects
CAS	Crash Analysis Database
CBR	California bearing ratio
CNMP	Construction Noise Management Plan
CPT	Cone Penetrometer Test
EBD	Enquiry by Design
ESA	Equivalent Standard Axles
GIS	Geographic Information System
GPS	Global Positioning Satellite
HAIL	Hazardous Activities and Industries List
HBRC	Hawke's Bay Regional Council
HDC	Hastings District Council
HCV	Heavy Commercial Vehicles
HIRDS	High Intensity Rainfall Design System
HPMV	High Productivity Motor Vehicle
HPTS	Heretaunga Plains Transportation Study, February 2012
LIDAR	Light Detection and Ranging
MOTSAM	Manual of Traffic Signs and Markings
MSL	Mean Sea Level
NES	National Environmental Standards
NZTA	New Zealand Transport Agency
PNGL	Palmerston North to Gisborne Line
PPF	Protected Premises and Facility
RCA	Road Controlling Authority
RL	Reduced Level

Abbreviation	Description			
RLTS	Regional Land Transport Strategy			
RMA	Resource Management Act (1991)			
SH	State Highway			
SSTMP	Site Specific Traffic Management Plan			
TCD	Traffic Control Devices			
vpd	Vehicles per day			
vph	Vehicles per hour			

1.4 Structure of this Document

This report is presented in six sections briefly described as follows:

Section 1 Introduction: Provides an introduction to the project and description of the overall project objectives.

Section 2 Background Information: Describes where the project fits within the HPTS and RLTS and summarises previous consultation and processes undertaken to develop the recommended alignment.

Section 3 Location: Provides a high level description of the route and key factors influencing the design.

Section 4 Road Design: Describes the key design elements, standards and constraints

Section 5 Project Construction: Describes a high level overview of how the project will be constructed and how constructions effects are to be managed

Section 6 Operation and Maintenance: Provides a description of long term operational effects of the project, ongoing maintenance and monitoring requirements.

Appendices: The following appendices are attached to this report:

Appendix A Technical Drawings – provides detail designed information for the alignment, typical cross sections, intersection design and bridge design (etc)

Appendix B Lighting Subcategory Evaluation – provides lighting design sub-category evaluation

Appendix C Hawkes Bay Regional Council Spill management plan

2. Background Information

2.1 HPTS

The 2012 HPTS was formally adopted by the Council in April 2012. The aim was to:

"ensure that people and goods are moved to/from and within the study area with the least cost and for the most benefit to the region's economy while enhancing its social and cultural fabric and environmental condition".

The HPTS informed the Hawkes Bay Regional Land Transport Strategy for 2012 - 2042 which identified the WAL as a priority project for the short term (2012-2017) programme.

For initial assessment purposes, the HPTS considered three options (referred to as Options 22, 23 and 24) for an improved link between Whakatu and Pakowhai Road. The aim of this link is to improve access for freight from the growing industrial area at Whakatu along the Expressway to the Port of Napier, as well as providing a link from the hinterlands to the Whakatu industrial area. The three options were evaluated against economic and environmental factors. Option 24 (indicated in yellow on Figure 1 below) was ultimately identified in the HPTS as the preferred option.



Figure 1: Route options identified in the HPTS

Option 24 was preferred because it provides the most direct route between the SH2/Napier Rd roundabout and Pakowhai Road than the alternatives, being more attractive for traffic from Havelock North and more closely aligned with the strategic objectives of the WAL.

2.2 Enquiry by Design (EBD) Process

To refine and develop route options based on a wider appreciation of values and impacts, an EBD process was initiated by the Hastings District Council. The EBD process was a collaborative, community driven design process to explore and test different design and development ideas and options based on a comprehensive understanding of local issues, opportunities and constraints. A working group was established for the process involving community members (including businesses, landowners and environmentalists), Council staff and consultants. The overall objective of this working group was to develop the WAL from an initial concept to a preferred route option ready for consideration by the full Council.

This process is more fully described in the Alternatives Assessment Report (EMS 2014b) which also includes a copy of the Enquiry by Design Working Group Report.

In summary, the recommendations from the EBD Working Group were:

"...that the Hastings District Council accept the refined Option 3 (as shown in Appendix J of the Working Group Report) as the preferred route option for the Whakatu Arterial Link. Option 3 is recommended because it provides:

- improved safety for the Whakatu community by drawing traffic off residential streets and around the Mangateretere School location;
- improved safety at the SH2 and Pakowhai Road intersections;
- better connectivity to the industrial area;
- better economics as indicated by higher BCR and FYRR values;
- good potential to incorporate cycle lanes and walkways;
- an attractive route from Havelock North to Napier and the airport; and
- clear sight lines and geometry.

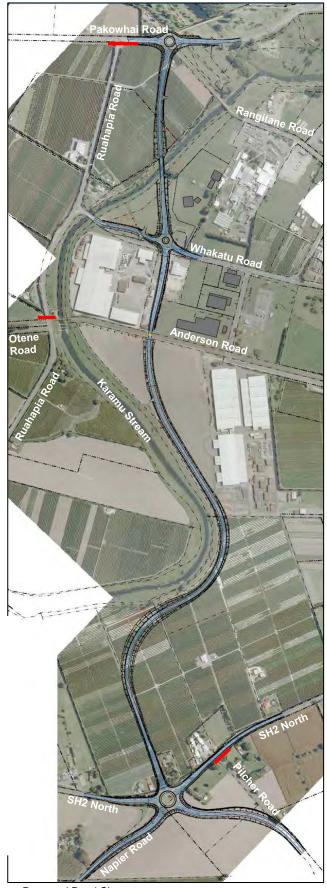
Given this recommendation, the Working Group also makes the following additional recommendations and comments to the Hastings District Council and it is requested that these are considered in any decision on the development of Option 3 for designation, detailed design and construction:

- Include an extended Anderson Road and roundabout on the Arterial;
- The extension of Anderson Road with a new bridge across the Karamu Stream to link to Otene Road and a strategic link through to the NE Connecter should be considered by HDC and the Regional Transport Committee;
- The design around the Pilcher Road / SH2 intersection is a key safety issue which must be well addressed in detailed design, particularly around the Mangateretere School;
- The design of the Pakowhai Rd arterial intersection is a key design issue and requires effective and safe linkages with Farndon Rd and Ruahapia Rd;
- Through design and traffic management, discourage the use of Farndon Rd as a heavy traffic route to Napier and the Port, but not to Awatoto;
- Any final design needs to look at maintaining efficient land use as much as possible and minimising the loss of fertile land;
- Maintaining good access to the Pakowhai Country Park will be an important design consideration for intersections in the Pakowhai / Ruahapia or Chesterhope Bridge area;
- Consider opportunities for discouraging the use of alternative, less-efficient routes (rat runs) and utilisation of closed roads for cycle / walkways; and
- If there are any road closures, road names will need to be altered to help with prompt identification and access for emergency services."

The Hastings District Council accepted the recommendations from the EBD Working Group and Option 3 has been confirmed as the preferred alignment to proceed to detailed design 7 August 2012.

The final alignment confirmed through detailed design is shown in Figure 2 below. This alignment includes design refinements developed in consultation with landowners, the EBD Working Group, and the broader community. This process is fully explained in the Alternatives Assessment Report (EMS 2014b).

Figure 2 Route Alignment



Proposed Road Closure

3. Location

3.1 **Project Location and Description of Route**

The WAL is located within the Hastings District and extends between Pakowhai Road and SH2 to the North East of Hastings Township. The adjacent land use is predominantly industrial with horticultural properties located towards the eastern and western ends of the alignment. The general location of the project is indicated in Figure 3 below. A detailed route alignment is shown in Figure 2, (Section 2 of this report).

opatield Rd Bay View Main North Ahuriri Napier Dartmoor Rd Onekawa Puketapu Rd State HW Greenmeadows Taradale Meeanee THE Rd Korokipo Rd 50 Clive tate Hwy Location of Whakatu Arterial Twyford 50A Iwood Ro Te Mata Mangateretere Rd Cino Flaxmere Frimley Whi Tuki Rd Stock Rd Hastings o Rd Parkvale Paki Paki Rd Havelock North Te Aute Rd Waiman RÓ

Figure 3: Indicative route location

The route is approximately 3,500m long and is orientated in a general north-west/south-east direction extending from the west at Pakowhai Road near the closed-off Rangitane Road, through to the intersection of SH2 with Napier Road to the east.

The alignment crosses the Karamu Stream 400 to 450m east of Pakowhai Road and from there continues on the northern side of this stream. The WAL intersects with one local road, this being Whakatu Road. The Whakatu Road intersection will be a 4 legged roundabout.

Whakatu Road is classified as a local road in the operative and proposed Hastings District Plan.

The existing rural environment has a posted speed limit of 80 km/h. This speed limit changes to 50km/h within the Whakatu urban area. On Anderson Road, this reduction occurs outside Whakatu Cold Store Limited.

The Palmerston North-Gisborne railway line (PNGL) crosses the alignment at an approximate mid-point (approximately 1,050 metres east of Pakowhai Road).

3.2 Affected properties

The properties from which land will be required for the construction of the WAL are outlined in the land requirement plan in the Notice of Requirement (HDC 2014a)

By way of summary, Table 1 lists affected landowners. The alignment crosses 14 lots owned by private land owners, with the remainder of the land being owned by either HDC, Hawke's Bay Regional Council or the Crown.

Property Reference Number	Claimant	Total Area to be Acquired(Ha) (road and severance)		
2	Wedd, Ward & Bell	0.0371		
3	Wedd, Ward & Bell	0.5548		
4	Wedd, Ward & Bell	1.3925		
5	Webb, Ward & Bell	0.8875		
6	Omahuri Orchards Limited	0.9547		
7	Mr Apple New Zealand Limited	2.2071		
8	Haley	0.0187		
9	Mr Apple New Zealand Limited	2.1616		
10	ENZA Group Services Limited	3.0735		
12	Apollo Pac Limited	0.9432		
13	Lucknow Holdings Limited	1.8590		
14, 15, 17, 18	Hawke's Bay Regional Council	0.7065		
16	Crown (River Bed)	0.1106		
19	Andrew Bryan Dillon	0.7625		
20	Lucknow Holdings Limited	1.2831		
22	Hawke's Bay Regional Council (River Bed)	0.0425		
23	Hastings District Council	0.0720		
24	Hawke's Bay Regional Council	0.0293		
25	Silverfern Farms	0.3480		
26	Road Reserve (Hastings District Council)	0.3378		

Table 1: Summary of Total Land Requirements

4. Road Design

4.1 Investigations informing design

4.1.1 Initial Geotechnical Testing

The following ground investigations were carried out:

- A total of 21 hand Augers including Scala penetrometers at various locations along the alignment; and
- Four truck mounted CPTs at the proposed bridge abutment locations, in which the data assessment also included a liquefaction assessment.

The CBR of the subgrade between depths of 250 mm to 1000 mm is typically below 4 along the majority of the proposed alignment. At certain locations the CBR trends towards a value of 1 below a depth of 1 m.

The water table is relatively high in this area.

Overall the quality and moisture content of the subgrade varies considerably along the proposed alignment.

These results show that the subgrade will require stabilisation in order to meet minimum road design requirements. It is also likely that the road pavement will require stabilisation.

Given the high water level in some locations, the road pavement will be elevated above ground to stay well clear of the water table. This will necessitate increased levels of fill and this has been reflected in detailed design drawings.

4.1.2 Pavement test pits and Laboratory Testing

Previous pavement testing had been carried out as part of investigations for improvements to the SH2 Napier Road Intersection. This data was reviewed along with the previous geotechnical investigations identified in section 4.1.1 above in order to develop an approach to pavement testing. It was determined that a total of seven test pits were required in virgin ground along the proposed route, with an additional two test pits to determine existing pavement materials on Pakowhai Road at the proposed intersection location.

Samples were collected at various locations and depths to enable laboratory testing which consisted of 14 Subgrade CBR tests compiling both natural and cement modified samples to determine the strength of the insitu material.

4.1.3 Bridge borehole investigations

A total of three boreholes were drilled to a depth of at least 20 metres at the anticipated bridge pier locations. Each location was marked and rationalised to allow access for the drilling rig.

Alluvial sand and gravels were encountered with competent material (i.e. having a Standard Penetration Test N value of 50 or more blows) being reached at depths of 11 and 12 metres. These results informed preliminary bridge design parameters, including a recommendation that piles are continual flight augered or bored cast in situ (with protective casing during excavation).

4.1.4 Test Pits for Fill Borrow areas and Laboratory Testing

HBRC have programmed widening works (for flood control purposes) for the Karamu Stream in the vicinity of the proposed WAL bridge. Through initial consultation with HBRC, it became

apparent that these widening works may provide a significant quantity of fill suitable for WAL construction. This could provide a practical and environmentally sound solution in that the volume of fill being trucked in to the site could be reduced.

In order to determine the suitability of this material for use as fill, a total of six 3-4m deep test pits were dug to build on the information gathered from the bridge bore holes in an area adjacent to the Karamu Stream. Samples were collected at various locations and depths from these test pits to enable the following laboratory testing to be carried out:

- Six UCS tests to determine strength parameters;
- Three compaction curves to determine the optimal water content for maximised compaction properties.

The laboratory testing confirmed the suitability of the material for use as fill material provided it was adequately dried before placement. The relatively free draining nature of this material indicates that this will not be a problem when coupled with suitable construction methodologies.

4.1.5 Topographical Survey

A topographical survey was undertaken by "The Surveying Company" using hand held GPS. A general 20m corridor was surveyed along the proposed alignment as developed through the Enquiry by Design process with additional areas surveyed in and around proposed intersections.

Generally cross sections were spaced every 20m to 30m for green field areas and increased in frequency for areas where topography changed and areas where existing roads were being surveyed. The survey was aligned to the NZGD HB Circuit, which has a recorded mean sea level (MSL) of 0.0m.

4.1.6 Trial Pavement

A 60m x 5m trial pavement was constructed to better gauge the actual response of pavement materials in the field. Significant field testing was carried out which allowed for a 10% reduction in the thickness of the subbase materials and a 40% reduction in the basecourse layers where a heavily stabilised subbase is used.

4.1.7 Pavement Design Inputs

All design and traffic estimates followed the procedures in Austroads Guide to Pavement Technology: Part 2 Pavement Structural Design (Austroads Publication No. AGPT02/10, Austroads, 2010).

The assumptions used to estimate the design traffic are detailed in Table 1. A conservative lane distribution factor (LDF) of 1.0 was used as it is expected that most heavy vehicles will travel in the left lane only. Pavement design traffic volumes calculated from the traffic data are shown in Table 2. This design focuses on the main alignment with the highest traffic volume.

	Pakowhai Rd End	Pilcher Rd End
Estimated HCVs (2015) – assume linear growth	2787	1810
AADT (2026)	15970*	10370*
Number of HCVs (2026)	2990	1920
Number of HCVs (2046)	3360	2120
LDF	1	1

Table 2 Design Traffic Assumptions - Whakatu Arterial

	Pakowhai Rd End	Pilcher Rd End	
ESA per HCV	1.8**	1.8**	
days per year	365	365	
Directional Split	50%	50%	
Design Life (years)	25	25	
TOTAL DESIGN ESA	26 Million	17 Million	

LDF = Heavy Vehicle Lane Distribution Factor

* AADT subtly different to final traffic model figures due to intersection configuration changes and pavement design not being recalculated

** Accounts for new HMPV vehicles (was 1.4 ESA per HCV before HMPV).

Table 3 details the traffic multipliers required for the different pavement material types.

Table 3- Traffic Multipliers used in CIRCLY Pavement Design Software

Material	Traffic Multiplier
Subgrade (vertical compressive strain)	1.17
Asphalt (tensile fatigue)	1.01
Cement Stabilised Aggregate (tensile fatigue) and foamed bitumen stabilised aggregate	2.65

4.1.8 Design subgrade CBR

Investigations with the scala penetrometer found areas of the site had low subgrade strengths of a CBR of 2%. In order to form a working platform to compact the overlying pavement layers the subgrade was stabilised with 2% cement. After stabilisation the subgrade was retested using the scala penetrometer and the result was a crust on the top of 100mm. In some cases the CBR increase to a value of 16%.

4.2 Pavement Design

The pavement design was developed by Dr Greg Arnold from Road Science with basecourse layers ranging from 210 - 220mm, stabilised (4% cement) SubBase (GAP65) of 270 - 330mm and cement stabilised (2% cement) subgrade 200 - 250mm.

MWH peer reviewed the design and in conjunction with GHD hosted meetings with Fulton Hogan to discuss the pavement design. Throughout these meetings HDC agreed to accept the risk on the pavement design based on Fulton Hogan's field experience in the Whakatu area.

The proposed pavement design under this risk transfer process is estimated to be 210mm of basecourse with GAP 65 to 270mm in areas where the pavement is on a fill of greater than 1m or where a CBR of 8 can be achieved through subgrade improvements i.e. geogrids or geotextile. In areas where the fill is less than 1m the GAP65 will be stabilised in addition to subgrade improvements with geogrids or geotextile.

4.3 Alignment detail

On the basis of the alignment developed through the Enquiry by Design Process and through inputs from initial geotechnical testing, detailed design of the road alignment was developed.

Geometric design of the alignment has been undertaken in accordance with the following standards and guidelines:

- Austroads Guide to Road Design (suite of standards), 2010
- NZS:4404.

- HDC Engineering Code of Practice (ECOP), 2011
- HDC Subdivision and Infrastructure Development in Hastings, Best Practice Design Guide, 2011.
- NZTA Traffic Control Devices (TCD) Manual and NZTA Manual of Traffic Signs and Markings (MOTSAM).

The WAL has been designed to function as a District Arterial Road based on HDC District Plan classification criteria.

As described in Section 3.1, the existing rural environment has posted speed limits of 80 km/h.

The design speed for the new arterial has been set at 80km/h as a result of the alignment determined through the EBD process. This is due to the radius of the two curves between SH2 and Anderson Road which have been designed to minimise the impact on productive land use activities which in turn has reduced the design speed of the curves.

4.3.1 Long Section

The alignment is 3.5 km long and is curvilinear in nature and roughly follows the course of the Karamu Stream. The tightest curve on the route is of radius 240 metres. The route is predominantly level with the exception of small gradients to assist water drainage and for the bridge approaches as well as the tie ins to existing road levels. The maximum gradient on the route is 1% on the southern approach to the bridge.

Appendix A provides detail designed information on the alignment, including technical drawings and plans.

4.3.2 Typical Cross Sections

The typical cross section for the WAL has been developed based on Austroad and HDC standards and is consistent with the existing road environment.

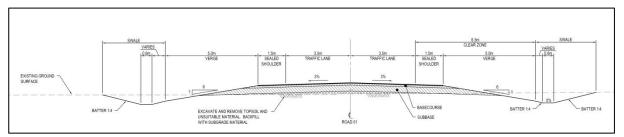
The arterial comprises of the following components:

- 1.5 m wide shoulder; and
- Two x 3.5m wide traffic lanes.

This results in a total carriageway width of 10m. Taking into account areas required for fill and stormwater treatment measures (refer to section 4.12) the total width of land required for the road is on average 36m.

Figure 4 below shows the extent of the typical cross section including the roadside swales. Clear zones are maintained on both sides of the alignment.

Figure 4: Typical cross section



The carriageway widens on the approach to the two terminal roundabouts (i.e. at SH2 Napier Road and Pakowhai Road) to two entry and two exit lanes from the roundabout. There is also some lane widening on the approach and exits to the Whakatu Road roundabout. Further details on these intersections are provided below.

4.4 SH2, Pilcher Road and Napier Road Intersection

The proposed WAL Intersection with Napier Road, SH2, and Pilcher Road is a dual lane, five leg roundabout. The majority of the WAL roundabout is located within an existing designation held by NZTA to accommodate a roundabout previously proposed for this location.

Various options were considered for the design and location of this roundabout. This process is discussed fully in the Alternatives Assessment Report (EMS 2014b).

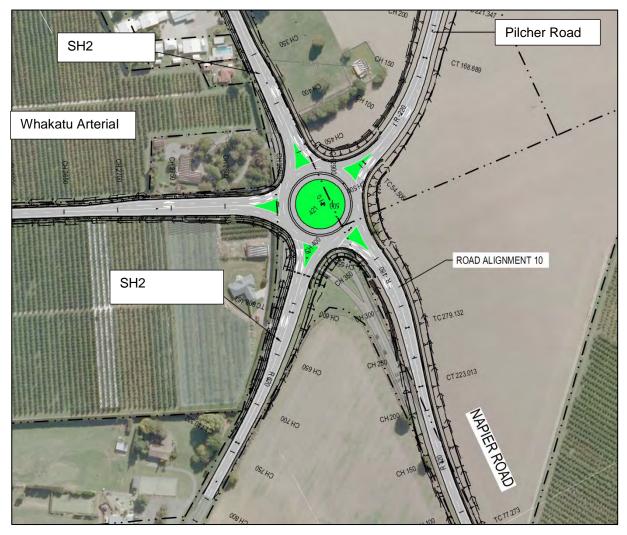
The WAL approaches the roundabout from the west with SH2 approaching in a general northsouth direction consistent with the existing SH2 alignment. The roundabout has a 30 metre radius central island, providing for an 18 m semi-trailer design vehicle.

The Napier Road approach requires a slight deviation from the existing alignment to provide separation to the SH2 southern approach. This results in an approach radius of 160 metres for the Napier Road arm from Havelock.

Land is required from properties located to the east of the intersection to accommodate the alterations to Napier Rd, the realignment of Pilcher Road and the roundabout itself. With the exception of the land required for the Whakatu Arterial road reserve, land requirements on the western side of this intersection are minimal.

The general layout of the intersection is shown in Figure 5.

Figure 5: Indicative layout of SH2 / Whakatu Arterial / Napier Rd Roundabout



4.5 Pakowhai Road Intersection

The Whakatu Arterial intersection with Pakowhai Road is a proposed three leg roundabout.

The roundabout is largely accommodated within the rest area located near the end of Rangitane Road. Rangitane Road does not currently intersect with Pakowhai Road. It will remain blocked off from Pakowhai Road and is therefore only accessible from the east. It is also noted that the Rangitane Road bridge is not suitable for conveying vehicular traffic and is slated for removal by HBRC.

A small amount of land in the Pakowhai Regional Park will be affected by the roundabout. The Proposed Conditions (HDC 2014d) include a requirement to mitigate the impacts of the roundabout on the Regional Park, including such matters as impacts on access and parking. A concept plan for achieving appropriate mitigation of these impacts has been developed in consultation with HBRC and is included as a schedule to the Proposed Conditions.

Pakowhai Road will retain its existing north-south alignment, with the Whakatu Arterial approaching from the east. The central island has a 20m radius, able to accommodate the 18 metre semi-trailer design vehicle.

The general layout of the intersection is shown in Figure 6. The revised park access as outlined in the concept plan included with the Proposed Conditions (HDC 2014d) is also shown.



Figure 6: Indicative layout of Pakowhai Rd / Whakatu Arterial Roundabout

4.6 Local Road Intersections

4.6.1 Whakatu Road

There will be one local road intersection on the arterial and this will be with Whakatu Road taking the form of a roundabout intersection with a 13m radius central island.

Whakatu Road currently provides the only crossing of the Karamu Stream from the south via Ruahapia Road. This bridge crossing will be maintained, however Ruahapia Road will be closed off east of the railway crossing near Otene Road and at the Ruahapia Road / Pakowhai Road intersection. Refer to discussion on road closures in Section 4.9 below.

The bridge and south-western end of Whakatu Road is therefore intended to only be used for property access including the Apollo Pac industrial development east of the bridge.

Whakatu Road will be slightly re-aligned on the approaches to the roundabout to ensure sufficient entry deflection and approach angles.

The layout of the intersection is shown in Figure 7.

Figure 7 Whakatu Road Intersection



4.7 Farndon Road Intersection

Works proposed at this intersection are not part of the WAL project, either as defined by the HPTS or as part of the notice of requirement or consents applications being lodged, and are not required for the efficient operation of the WAL. However, proposed improvements are outlined here because the EBD process included a specific recommendation that the safety of this intersection be improved as part of the new intersection of the WAL with Pakowhai Road. In response to this recommendation, a design is proposed in this document and it is understood that these improvement works will be undertaken by HDC as part of a safety upgrade.

Four options for the Farndon Road / Pakowhai Road intersection have been assessed, being:

- 1. Intersection becomes a three arm roundabout (40m internal diameter);
- 2. Right turn out of Farndon Road is prohibited, a channelised southbound merge is added. Left and Right turn into Farndon Rd remains unchanged;
- 3. Right turn out of Farndon Rd is prohibited, existing left turn out of Farndon is relocated to current right-turn lane and a solid median introduced to channelize movements on Pakowhai Rd; or
- 4. Combination of 2 and 3 above.

The roundabout option was not considered favourably as it would interrupt (slow down) through traffic along Pakowhai Road and could not meet site distance criteria due to the close proximity of the bridge and vertical grade issues.

Options 2, 3 and 4 all allow for uninterrupted flow from the new arterial road to the expressway, whilst providing for improved safety for access to and from Farndon Road.

Option 4 has ultimately been assessed as the option that best meets project objectives and the expectations of the EBD working group. This option utilises the existing left turn slip facility from Pakowhai Road onto Farndon Road.

A design has been developed to fit within the existing road boundaries, preventing land requirement. This has required the inclusion of a retaining wall and guardrail that may require visual mitigation. Refer Figure 8 below:

Figure 8: Farndon Road Intersection Improvements - Preferred Option 4 Layout



4.8 Rail Level Crossing Design

The Palmerston North Gisborne Line (PNGL) runs in an approximate north-south direction though Whakatu. The WAL will require a new crossing of the PNGL approximately 300 m east of the proposed WAL / Whakatu Road roundabout.

In consultation with KiwiRail, the new rail level crossing (New Crossing 163.80 km) will be developed with the following conditions (to be confirmed through final agreement with KiwiRail):

- The new crossing is to be held under Deed of Grant.
- The layout of the new crossing must be approved by KiwiRail.
- A new half-arm barrier automatic alarm system is to be installed.
- The crossing is to be constructed by KiwiRail to current KiwiRail standards.
- No costs of construction of the crossing or installation of the alarm system will fall on KiwiRail.

In addition, it is a requirement of KiwiRail that in order to provide for a new crossing of the PNGL, an existing crossing in the area must be closed. This is discussed below.

4.9 Closure of the Ruahapia Road Level Crossing

There are currently two existing level crossings operated by KiwiRail in the Whakatu area; one on Ruahapia Road and one at the intersection of Whakatu Road and Anderson Road in Whakatu.

KiwiRail policy requires that the approval of a new rail crossing does not result in a net increase in the overall number of rail level crossings in order to avoid safety and operational impacts. In order to achieve this result, either the Ruahapia Road level crossing or the Whakatu Road level crossing must be closed.

The closure of the Ruahapia Road level crossing has been considered as the preferred option for the following reasons:

- Discourages the use of Ruahapia Road, reducing traffic and improving safety for the community.
- Removes the conflict at the existing level crossing that currently has a substandard road alignment from the southeast.
- The Whakatu Road / Anderson Road rail crossing is required for internal traffic movements within Whakatu; its closure would significantly impede connections from Whakatu to the WAL.
- Some local traffic, i.e. Ruahapia Road residents that would traditionally use the Otene/Ruahapia/Whakatu Roads to access Whakatu and beyond, would need to divert and increase the travelled distance to reach their destination. However, the majority of traffic will travel shorter distances with the WAL in place.

In addition to closing the Ruahapia Road rail crossing, Ruahapia Road will also be closed at the intersection with Pakowhai Road. This is a historically unsafe intersection, and will no longer be required. Through traffic will use the WAL and access to existing properties on Ruahapia Road north of the rail crossing will be provided via the existing Whakatu Road Bridge.

HDC will need to initiate a formal road closure process under the provisions of the Local Government Act or Public Works Act in order to give effect to these closures.

The consultation and options analysis process undertaken in considering the closure of the Ruahapia Road rail crossing is outlined in more detail in the Alternatives Assessment Report (EMS 2014b)

4.10 Cycleways

The WAL will cross the existing Hastings-Clive cycleway in Whakatu. Access across the WAL is proposed via an underpass constructed on the line of the existing limesand path. The underpass will be constructed to maximise sight lines and lighting.

Pedestrian and cycle movements are catered for in the footpath and cycle way on the WAL bridge. The WAL road shoulders provide space for cyclists along the remainder of the route however it is anticipated that these road users will use Ruahapia Road and Whakatu Road rather than the WAL.

It is understood that HDC has plans to promote the use of Ruahapia Road for cycle use and will provide more off road connections to the adjacent network.

4.11 Karamu Stream Bridge Design

The WAL crosses the Karamu Stream once near the northern end of the project area. The design of this crossing is a key element, given inherent engineering, environmental and cultural considerations.

4.11.1 Karamu Stream Management Approach

Bridge works have been designed with guidance from the Cultural Impact Assessment (Ipurangi Developments 2014) and with input from HBRC.

The Cultural Impacts Assessment includes the following recommendation regarding bridge design:

Recommendation 5: The proposed bridge is designed and constructed in a way that does not obstruct the flow/ mauri of the Karamu stream.

The HBRC have various works proposed to maintain adequate flow and reduce the risk of flooding of the Karamu Stream. The stream has issues with weed collecting on any obstructions within the waterway which increase the risk of flooding. Works currently proposed by HBRC for improvements in this area include:

- Removal of the Rangitane Road footbridge and floodgates;
- A long term plan to work with HDC to remove the main trunk sewer mains that cross the stream at Otene Road; and
- Construction of a widened flood channel from the Whakatu Road bridge to the Rangitane pedestrian bridge.

Comments from HBRC in relation to the WAL bridge include:

- Piers must be of a suitable design profile that prevents debris, particularly weeds, from catching on the upstream side during flooding events;
- HBRC will consider abutments intruding slightly beyond the channel profile; and
- The Q100 flows are estimated at 135 cubic metres per second.

These considerations have informed a bridge design that avoids disturbance of the wet stream bed.

4.11.2 Constraints

The design minimum soffit level of the bridge deck required by HBRC is at a reduced level (RL) of 6.2m above a mean sea level (MSL) of 0 metres.

HBRC are planning 35 metres of channel widening in the area of the bridge to provide for a flood channel at a RL of 2.0m. This requires a bridge length of approximately 72 m between abutments. HBRC have allowed the introduction of an intermediate pier within the proposed widened flood channel, reducing the clear span to approximately 40m. The stream cross section at the bridge location is shown in Figure 9.

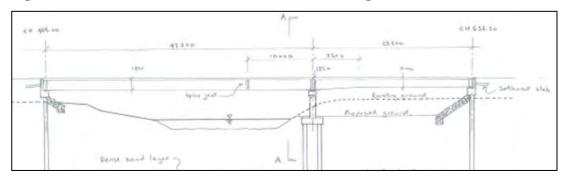


Figure 9 Karamu Stream Cross Section with Bridge

4.11.3 Design Parameters

The bridge will be designed in accordance with the NZTA Bridge Design Manual, 2013.

No construction activities are to take place in the wet stream bed.

4.11.4 Design Loadings

The bridge will be designed for full HN-HO-72, with full HPMV capabilities.

Seismic loading will be allowed for in the design.

4.11.5 Design Basis

This bridge is intended to provide for two 3.5m traffic lanes and a 1.8m shoulder on each side of the carriageway. Guard rails will be provided on each side of the carriageway to restrain errant vehicles. There is a 2m wide footway/cycleway and a pedestrian parapet behind the guard rail on the north side as shown on the Figure 10 below.

The design consists of a composite concrete deck on steel girders supported on reinforced concrete piers. The piers will be supported by augured cast in situ reinforced concrete piles, joined to the piers by a rectangular concrete pile cap.

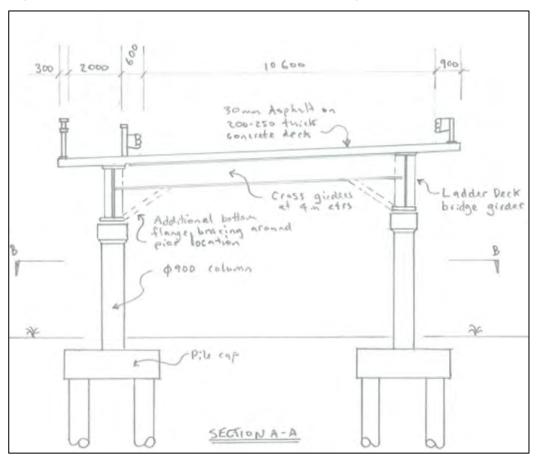


Figure 10 Cross Section of Karamu Stream Bridge

4.11.6 Construction

The contractor appointed through the tendering process will be required to provide details of the bridge construction process through detailed design and the final approved Construction Environmental Management Plan (GHD 2014i). It will be necessary for the tenderer to demonstrate that the construction will not create adverse effects in the Karamu Stream.

An anticipated construction process is outlined below to assist with an assessment of environment effects.

• The construction sequence would involve first augering the hole for the pile and then inserting steel casing to prevent collapse. Next the reinforcement is inserted before the

concrete is poured. Whilst the concrete is being poured the steel casing is extracted. All piles are located outside of the wet stream bed.

- After the piles are completed the concrete pile caps are then cast in situ forming a
 platform on which to construct the concrete piers. These will be cast in situ using
 temporary shuttering and temporary supports where necessary. Once the concrete has
 gained sufficient strength the shuttering and supports will be removed.
- The steel girders for the bridge will be craned into place from cranes at suitable locations (within dry land, i.e. no interference with the running water). Whilst fabrication of the girders will be off site some assembly of components prior to lifting into place may be required on site. After the girders are in place the concrete sections forming the deck will be craned in. Depending on the final design a top slab may be cast in situ on top of the concrete panels.
- Finally water proofing followed by road pavement materials and surfacing together with parapets and guard railing will be installed.

4.11.7 Design Basis

The basis of the design is to allow construction of the bridge without interfering with the stream channel. The use of precast concrete or steel girders has been identified as a solution that avoids the need for temporary works in the stream bed and less risk of pollution incidents.

4.12 Stormwater Design

4.12.1 Stormwater Configuration

A Stormwater Management Plan (GHD 2014g) has been developed and is provided with the RMA application documentation. It outlines the proposed approach to stormwater management during the operational phase of the WAL, including specifying minimum design and operation requirements. A supporting Erosion and Sediment Control Plan (GHD 2014h) is also provided to manage the effects of surface water runoff during soil disturbance activities associated with construction.

The management plans present a low impact design that utilise swale drains on both sides of the road to collect and convey stormwater runoff. The swales discharge into the existing roadside drains at the tie-ins with the existing road network and into the Karamu Stream where the WAL is in close proximity to the stream.

The swales are designed to allow for infiltration to reduce peak discharge flows and to provide stormwater treatment.

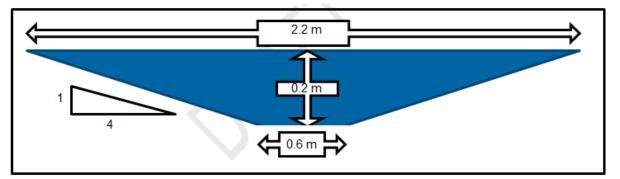
The proposed swale cross section was derived using the following parameters:

- Maximum side slopes of 1:4 (H:V), which is within the 1:3 maximum as recommended by NZS 4404:2010;
- 0.6 m minimum base width to create a large surface area to increase the extent of infiltration and filtration;
- Minimum depth of 0.2 m. Depths less than this are generally not recommended as a minor obstruction has the potential to completely block the swale.

These considerations resulted in a minimum cross sectional profile as illustrated below (the sketch is not to scale). The swale will be deeper and therefore wider where required to:

- Provide sufficient flow capacity; or
- Due to the longitudinal grade in relation to the surrounding ground levels.

Figure 11 Minimum Swale Cross Section



4.12.2 Hydrology and Hydraulic Analysis

The extent of stormwater runoff for each swale was estimated using the rational method as outlined in The New Zealand Building Code Clause E1/VM1 for a 10-year design storm return period. The area's rainfall intensity was determined using NIWA's High Intensity Rainfall Design System (HIRDS). A 10 minute time of concentration was estimated for the proposed swales.

The following runoff coefficients were used in the calculations:

- 0.3 for grass covered areas;
- 0.9 for the sealed carriageway.

The swales were then sized to convey this flow using Mannings equation. A longitudinal grade of 0.2 % was assumed for all swales. This is the minimum grade recommend and due to the flat topography it is likely that most of the swales will be at this minimum grade.

4.12.3 Culverts / Pipes

The culverts will be sized so that they do not cause flooding of the upstream swales. It is proposed that culverts crossing the roads will generally have NZTA approved traversable wing walls (complete with grate) on either end.

Where the topography allows, the inlet to the discharge pipes under the Karamu Stream stopbank will be via a sump type configuration using scruffy domes and appropriately sized manholes. The top of the manholes will be at the invert of swale. Where the topography is such that this design is not achievable, a similar inlet structure to the road culverts will be used. Both of these systems have grates which are selected to prevent gross pollutants entering the Karamu Stream.

The outlets into the Karamu stream will be designed to minimise the impact to the stream bed and to reduce the risk of scouring. This will most likely consist of a normal concrete headwall with a riprap type system if high flow velocities are likely. To maintain flood protection, WaStop valves (or similar type check valves) will be required where the pipes pass under the stopbank.

There will be 7 outlets into the Karamu Stream.

4.13 Street Lighting Design

An initial street lighting design has been undertaken covering all intersections along the proposed alignment as well as the mid-block section between the Whakatu Road intersection, and the rail level crossing.

The required lighting level has been evaluated with a copy of the lighting design sub-category evaluation provided in Appendix B. The rating value of 110 is covered by both the V2 and V3

categories. V2 was agreed given that the expected increases in traffic in the future, as well as other demands on drivers including the rail level crossing that will eventually lift the required design level.

The initial design incorporates the Kaos light fitting. HDC are progressively replacing old fittings with the Kaos fittings. The Kaos fittings are acknowledged as a high calibre fitting with very limited light spill and glare technology and efficient light projection. The bulbs are still high pressure sodium; however there is new LED technology that is progressively becoming more mainstream, which could be considered. Potential benefits include lower power consumption and lower replacement frequency.

All new cabling for the street lights will be underground. Intersection lighting will generally be on individual circuits each requiring a transformer box that will draw electrical feed from the nearest source. The transformers will likely be stand alone, and mounted at ground level close by in a safe location.

All lighting has been designed to be mounted on the outside of the intersection on single new 12m high light columns, i.e. there are currently no lighting columns proposed for the internal roundabout islands. This makes access straight forward reducing future maintenance costs.

Table 4 provides a summary of the design.

Table 4: Street Lighting Design Summary

Location	Lighting Design Summary
Pakowhai Road Roundabout	17 new lights and 800m new cabling
Whakatu Road Roundabout	13 new lights and 650m new cabling
Napier Road SH2 Roundabout	30 new lights and 2200m new cabling

5. **Project Construction**

5.1 Construction Environmental Management Plan

All construction works are required to be carried out in accordance with a certified Construction Environmental Management Plan (CEMP) (refer Proposed Conditions HDC 2014d).

A draft CEMP (GHD 2014i) has been developed and is included with the RMA application documents. The CEMP is required to be updated by the contractor appointed through the tendering process in accordance with the proposed construction methodology. The updated CEMP must be certified by HDC and HBRC prior to construction commencing. The CEMP includes objectives and specific performance criteria that are required to be met through the construction phase such that environmental requirements are met.

5.2 **Project Construction Schedule**

This section is intended to provide high level information on the anticipated construction of the project to provide sufficient detail to assess the potential environmental effects and identify any necessary measures to avoid, remedy or mitigate those effects.

The majority of the WAL will be constructed off line (i.e. not alongside or on top of an existing road) through the industrial and horticultural land described in Section 3.

The critical elements in terms of managing effects on road users will therefore be the intersections with existing roads, where the staging of construction will need to be carefully considered in order to maintain traffic flow and minimise adverse effects on existing road users.

The other major considerations are maintaining property access, particularly to the industrial properties located around Whakatu Road, and minimising impacts on activities such as orcharding and pack house activities that are more sensitive to potential construction impacts during certain times of the year.

Construction of the project is expected to take around 18 months (or two construction seasons) with works carried out concurrently at various locations to optimise the construction process while seeking to minimise disruption to existing road users, land owners and the local community. The construction period is a guide at this stage. As the majority of the project will be carried out off line there is flexibility around the scheduling of work with opportunity to condense or extend this construction period.

It is envisaged that the construction of the main alignment can be undertaken concurrently with the key aspects described below and not be opened to traffic until the intersections are completed.

The key aspects of the construction are considered to be:

- SH2 / Napier Road roundabout;
- Karamu Stream bridge;
- Pakowhai Road roundabout;
- Whakatu Road Roundabout;
- Railway crossing;
- Property access particularly at Whakatu Road (Apollo Pac).

The contractor appointed through the tendering process will be required to finalise the CEMP which will include an outline construction programme identifying and sequencing all the key parts of the Contract Works. This baseline programme shall clearly indicate which parts of the works are to be under construction at any time and the total planned duration of each part. The total planned duration shall include all reinstatement works, including sealing, where this is required. This programme will be required to be updated on a regular basis during construction to gauge progress against the baseline programme.

The specific comprehensive construction methodology will be identified by the contractor in the final CEMP. The contractor will specify how the contract works will be delivered on time and to the standard and requirements specified in the Contract Documents. The methodology must clearly state how the work is to be carried out with minimum disruption to the users of the businesses in the project area and the public and indicate how they will ensure that the site is maintained in a safe and useable condition during the period of the work and any close down periods. This methodology will also demonstrate how the contractor will meet the environmental standards that may be identified in the environmental assessment and consenting process.

5.3 Construction overview

Enabling works

Early works will include the relocation or protection of services such as electricity and telecommunication services and privately owned assets such as affected infrastructure in the orchard blocks (refer Productive Land Use Impact Assessment, AgFirst, 2014). Throughout

property negotiations the early planting of shelterbelts could also be agreed and could form part of the enabling works.

Construction of site access roads particularly for construction of the bridge from the west and east will also be completed, together with the construction of the sedimentation ponds in accordance with the Erosion and Sediment Control Plan (GHD 2014h). The construction access roads will be located over the general alignment to minimise the effects on private land owners during construction, and the areas set aside for sedimentation ponds are included within the land proposed for designation to provide certainty for the construction process and landowners.

Main Alignment

As described above the construction scheduling of the main alignment will be relatively flexible due to it generally being undertaken off line. It is however envisaged that these works will be carried out concurrently with the major intersections.

A consideration for the construction of the main alignment is maintaining property access, particularly to the Apollo Pac Ltd industrial site whose existing access is located on the alignment of the arterial road. Construction methods in relation to property access are discussed further in Section 5.5.

Another key consideration is the impact on orchard blocks and processing facilities during critical harvest periods (refer Productive Land Use Impact Assessment, AgFirst, 2014).Construction activities will be programmed to avoid impacts on these properties during critical times, with the specific approach confirmed in the final CEMP and informed by landowner consultation.

The bridge construction will be scheduled during the initial phases of the construction period and is expected to take between 9 to 12 months. Works on the bridge could begin following the construction of the temporary access roads.

As discussed in Section 4.1.3, the WAL will source fill by undertaking planned flood channel widening on behalf of HBRC in the vicinity of the bridge. The fill resulting from these works is likely to be used for the construction of embankment fill on the project. No construction is proposed in the stream bed. The contractor will need to provide adequate protection on and around the bridge works to ensure no materials fall into the stream and pollute the water.

There is opportunity to locate a site compound in the area north of Whakatu Road for use during construction of the project.

Water quality effects during construction will be managed in accordance with the Erosion and Sediment Control Plan (GHD 2014h)).

Traffic Management

A Traffic Management Plan (GHD 2014j) has been prepared and included with the RMA application documents.

As the majority of construction work will take place offline (i.e. not connected with existing public roads) traffic impacts will be kept to a minimum. The three major intersections of the WAL are the most complex in terms of traffic management, and construction should not be programmed such that vehicle delays occur concurrently.

By careful staging of the works local access to affected properties can be maintained and detours kept to a minimum.

After award of the contract the contractor will be required to prepare SSTMPs for each stage and section of the works. These will form part of the Traffic Management Plan and will cover in detail items such as the level of traffic signs, lane widths, measures for pedestrians across the works and detour routes. The contractor will not be able to put any traffic management measures in place without the prior approval of either Hastings District Council or NZ Transport Agency as the RCA appropriate for the section of road affected.

5.4 Construction workforce

The construction workforce will be appointed by the contractor. The contractor appointed for the WAL will be required to nominate the following key personnel:

- Contractor's Representative
- Contract Manager
- Senior field supervisor(s)
- Quality Manager
- Environmental Manager
- Health & Safety Manager
- STMS Staff.

5.5 Hours of construction work

Construction will typically be undertaken between the hours of 7:00am and 6:00pm. At the three main intersections major works will be undertaken outside of peak periods to minimise traffic delay whenever possible. Hours of operation will be specified in the CEMP and SSTMPs for acceptance by the HDC.

KiwiRail has indicated that the construction of the new railway crossing will be undertaken by KiwiRail, which will ensure that any impacts on the rail network can be managed directly and avoided.

5.6 **Property access**

During construction, access will be affected to some properties. The impacts are expected to be caused by one of the following:

- Worksites blocking direct access or delaying access to properties;
- Access being within a road closure; or
- Property access being on a nominated detour route.

SSTMPs will contain details identifying the proposed mitigation measures for all relevant impacts on the properties for acceptance by HDC. Consultation with the owners and occupiers of all affected properties will be undertaken prior to the SSTMPs being submitted to HDC.

One of the most challenging access issues exists at the Apollo Pac site which is located off Whakatu Road in the vicinity of the new Arterial alignment. Access to this busy industrial site will be directly affected by the new road. To ensure no business disruption, the section of WAL through this site will only be constructed once an alternative access is established.

Another key issue is to avoid impacts during key harvest periods (AgFirst 2014).

The SSTMPs will provide specific provisions for access to affected properties. Consultation will also be undertaken through the development of these plans to ensure the residents/business have appropriate access to meet their needs, for example with respect to heavy vehicle provisions. It is expected that all access issues can be effectively managed through the development and implementation of SSTMPs.

5.7 Temporary facilities

5.7.1 Site Offices and Compound

An area to the north-west of Whakatu Road has been identified as a suitable location for the contractor's compound and site office. It is a decision of the successful contractor on where to site their site offices and site compound and this will be confirmed in the final CEMP. The contractor may negotiate the use of private land with the land owner during the tender process.

Services such as electricity and water to the site compound will be required and arranged by the contractor.

5.7.2 Construction laydown and stock pile areas

In addition to the main site compound and offices the Contractor may need to allow for laydown areas for construction plant and materials at the various works that may be ongoing at any one time. The number and locations of these will not be confirmed until a Contractor has been appointed.

5.7.3 Staff amenities

Staff amenities are to be provided at each work site on the project. Potable water for domestic use by the construction workforce will be transported to the construction sites. Port-a-loos will be used whereby sewage will be transported off-site and disposed of in a lawful manner by a contractor

5.8 National Environmental Standards on Air Quality

The National Environmental Standards (NES) relating to air quality during construction are expected to be met by the contractor. The construction activities are not expected to lead to adverse air pollution that exceeds the levels stipulated in the standards.

Dust suppression is to be addressed through a Dust Control Management Plan that will be prepared in conjunction with the CEMP. In particular, the effects of dust on fruit production during critical harvest and packaging periods must be mitigated.

5.9 Earthworks

Construction of the WAL will result in the removal of approximately 26,200m³ of topsoil and approximately 28,880m³ of cut to waste. All excavated material is to be disposed of offsite in accordance with any relevant District Plan requirements and any subsequent earthworks' consents granted to the contractor.

In order to construct the WAL there will be an overall importation of soil for embankment construction. It is estimated that 50,000m³ of fill will be able to be sourced through the completion of available HBRC's flood alleviation works in the Karamu Stream. The remainder of the structural fill is likely to be sourced locally within a reasonable area.

5.10 Potential impacts from construction

The main impacts from construction activities are expected to be:

- Dust
- Vibration
- Noise
- Heavy Commercial Vehicles (HCVs)

- Erosion and deposition of sediment
- Water Quality
- Pollution/Accidental Spillages.

These measures are discussed below.

5.10.1 Dust/Air Quality

For the construction phase potential sources of airborne dust include:

- wind-blown dust from exposed surfaces;
- wind-blown dust from stockpiles of dusty materials; and
- dust caused by vehicle movements on sealed or unsealed roads

In relation to potential for odour impact potential sources are from:

- Introduced contaminated materials; and
- Insitu soils with high organic content that have potential to decompose on contact with air.

As part of the CEMP, the successful contractor is required to provide a Dust Control Management Plan (DCMP) for certification prior to construction commencing. The DCMP shall include, but need not be limited to, the following:

- a) Description of the construction works and potential sources of dust;
- b) Periods of time when emissions of dust might arise from construction activities;
- c) Identification of any sensitive land uses that may potentially be adversely affected by emissions of dust from construction activities;
- d) Measures for minimising and managing dust emissions;
- e) Methods for monitoring dust during construction, including visual inspections of dust sources and dust generating activities, visual inspections of management measures, checking weather forecasts and observing weather conditions;
- f) Methods for undertaking and reporting on the results of daily inspections of construction activities that might give rise to dust; and
- g) Methods for receiving and responding to complaints about dust emissions.

The following measures are proposed to be incorporated in the DCMP to ensure that any shortterm actual or potential adverse effects on from dust generation can be avoided, remedied or mitigated:

- Initial vegetation removal should be limited to that necessary for construction purposes and carried out in discrete sections to avoid large sections of exposed earth;
- Vehicles running over exposed areas, including those that are in the process of becoming formed embankments have the potential to remobilise dust. In addition to dampening down with water, construction site speed limits should be implemented. The draft NZTA erosion and sediment control document (NZTA 2010) contains information about dust control (NZTA 2010, section 8.3.6);
- The number of stockpile areas should be minimised and provided with covers and / or shielded to prevent wind mobilising dust from the pile. Stockpiles may require water spraying and the loading and accesses should be hard paved and swept / damped

down on a regular basis. Stockpile areas should be reinstated following the conclusion of construction in accordance with the landscape philosophy statement; and

 Areas affected by construction should be re-vegetated as soon as each construction section is completed in accordance with the landscape philosophy statement. The draft NZTA erosion and sediment control document (NZTA 2010) contains information about soil stabilisation thorough topsoiling and grass seeding (Section 8.3.1), hydroseeding (8.3.2), mulching (8.3.3) and turfing (8.3.4).

It is considered that adoption of these measures will ensure that any actual or potential shortterm effects on amenity from dust generation will be minor, and that no significant long-term effects on amenity from dust generation will arise.

5.10.2 Vibration

The construction will be in close proximity to industrial sites, some of which involve sensitive processing equipment. ENZA in particular have highlighted concerns about vibration on their pack houses and operations therein. This has been investigated in the Noise Assessment (Hegley 2014).

A Construction Noise and Vibration Management Plan shall be prepared in conjunction with the CEMP (refer Proposed Conditions HDC 2014d) to manage these effects.

5.10.3 Noise

The requirements for construction noise are specified in NZS6803:1999: Acoustics-Construction Noise which sets maximum noise levels. The Noise Assessment (Hegley 2014) outlines the potential effects of construction (and traffic) noise associated with the WAL.

Plant commonly used for construction including motor scrapers, excavators, graders, rollers and pavers have been used to predict noise levels at each of the PPFs used for the Operational Noise assessment.

Typically, successful mitigation options include:

- Use smaller, quieter plant;
- Limit the use of some noisier equipment to the most exposed properties; and
- Use of screening.

The assessment of noise from typical construction activities has shown that some mitigation to the noisier plant may be necessary when working close to the most exposed properties.

There are practicable options for achieving the reductions required in order to meet the requirements of NZS6803:1999. However, due to the large number of variables that affect construction noise, the only practicable method of managing these effects is by the preparation of a Construction Noise and Vibration Management Plan by the successful contractor that is to be submitted for approval by HDC.

5.10.4 Impact of Construction HCVs on Roading Network

During periods of earthworks, particularly construction of the road embankment, there will be significant numbers of HCVs using the roading network. The frequency and duration of these movements will be dependent on the contractor's programming of the works. It is expected that 50,000 cubic metres of soil excavated as part of the flood defence works may be available thus reducing the distance travelled and numbers of construction HCVs travelling along the roading network.

The routes and frequency of HCV movements particularly during earthworks will need to be addressed by the contractor in the CEMP.

5.10.5 Sediment and Erosion Control

The removal of ground cover, earthworks and associated activities during construction may lead to the generation of sediment laden runoff which may affect water quality and affect adjacent agricultural areas. Appropriate mitigation measures are required to ensure that the effects of any sediment discharges are only minor.

A Sediment and Erosion Control Plan (GHD 2014h) has been prepared and is included with the RMA application documentation.

The Contractor will be required to comply with the requirements of the Sediment and Erosion Control Plan.

It is expected that the contractor will effectively programme the works so as to minimise the amount of un-stabilised ground exposed at any one time. The erosion and sedimentation measures are to be appropriately maintained throughout the construction works until the ground has been stabilised to ensure they function effectively.

5.10.6 Pollution and Accidental Spillages

The contractor is expected to provide measures to prevent accidental spillages during construction activities, particularly from oil and diesel storage tanks, entering watercourses. The approach taken will be included in the Contractor's final CEMP however it is noted that HBRC have developed a generic spill management plan. This is included as **Appendix C**. The final CEMP will incorporate the relevant provisions of this plan to ensure that appropriate management approaches are in place to address any spillages.

6. Operation and Maintenance

6.1 Operation

This section provides a summary of the maintenance issues and further operational features of the arterial road that have not been covered in the preceding sections of this report.

6.2 Maintenance

The long term maintenance requirements of the completed works are:

- Traffic services cleaning and repairs to road signs and lights, renewal of road markings;
- Railway crossing inspection and testing of signalling and barrier equipment;
- Road pavement resealing of road surface (between years 7 and 10) and rehabilitation of road pavement (between years 14 and 20) in future years;
- Guard Rails cleaning, inspection and repairs to guard rails including terminal ends;
- Bridge Structures General and Principal Inspections, repainting and graffiti removal;
- Drainage catchpit, swale, culvert and drainage channel clearing; and
- Landscape mowing of berms, trimming and replacing plantings.

The maintenance requirements will be dependent on the final construction design and will be detailed in the Asset Owner's Manual to be produced on completion of the project.

The responsibility of the new arterial road will lie with Hastings District Council as the Road Controlling Authority for the local road network whilst SH2 and the Pilcher Road roundabout (excepting the local road approaches) will be State Highway with NZTA as the RCA. The exact maintenance boundaries are to be determined.

With the exception of the level crossing barriers, associated signalling and track work which are the responsibility of KiwiRail, the above items will be the responsibility of the RCAs.

6.3 **Potential impacts from operation and maintenance**

Following construction, the ongoing operation and use of the Whakatu Arterial has the potential to cause environmental, social and economic impacts. The following potential impacts have been considered:

- Traffic and economics
- Productive land use
- Noise
- Vibration
- Landscape, natural character and amenity
- Natural hazards
- Stormwater and surface water runoff
- Ecology and water quality
- Cultural heritage and archaeology
- Social-cultural effects.

Brief outlines of these issues and their potential effects and the steps taken to avoid, remedy or mitigate them are outlined below. This information is intended to support and inform a full assessment of environmental effects as reported in the AEE technical assessment reports provided in Part C of the RMA application documentation.

6.3.1 Traffic and Economics

Crash Rates

The WAL is designed to current safety standards and is expected to result in a reduction in crash levels on the wider network as traffic diverts on to the new road. The existing five year crash records from the CAS database together with the crash rates and crash prediction models in Appendix A6 of the NZTA's Economic Evaluation Manual (EEM) have been used to determine the potential changes in crash rates over the study area as a result of the new arterial road.

Output from a refined HPTS traffic model has been used to analyse annual daily traffic (ADT) flow data and highlight key links and intersections that experience the most significant change in traffic volumes in the 2009, 2026 and 2046 road networks. In order to focus on the major accident costs and savings, those links that experience an approximate 10% change in volumes have been captured in the assessment. A minimum ADT threshold of 500 vpd was also applied to the selection criteria to put a sensible limit on the number of links and intersections analysed.

Overall, 22 intersections and 34 mid-block sections were analysed totalling approximately 60 km of road. The level railway crossings at Elwood Road, Ruahapia Road and Whakatu Road were also included.

The results of the analysis are summarised in the table below:

Table 5 Accident Cost Summary

Accident	Do Minimum			Project		
Cost	2009	2026	2046	2009	2026	2046
Mid-Block	\$7.11M	\$8.96M	\$10.26M	\$6.35.M	\$8.34M	\$9.72M
Intersections	\$2.66M	\$2.92M	\$3.15M	\$1.77M	\$2.01M	\$2.17M
Total	\$9.77M	\$11.88M	\$13.41M	\$8.12M	\$10.35M	\$11.89M

Mid-block crash savings mainly accrue from a significant amount of traffic transferring from local roads onto the safer arterial. The introduction of safer intersection controls (i.e. provision of roundabouts) for traffic travelling to and from the Whakatu industrial area also results in positive safety benefits.

Following the opening of the WAL crash records will need to be periodically reviewed to ensure the crash benefits are being realised. Should the post opening review of crash records indicate that the crash benefits are not being achieved, or if there are any safety issues with the new road, these can then be identified and consideration given to appropriate remedial measures.

Traffic Flows

From the HPTS traffic model the predicted changes in traffic flows from the existing network, (Do Minimum), to the proposed network with the new arterial road in 2026 have been determined and are shown in Table 6 below.

Road and location	Do Min	Project	Change
Whakatu Arterial (southern end)	-	9,750	+9,750
Whakatu Arterial (northern end)	-	16,750	+16,750
Ruahapia Road (southern end)	3,000	500	-2,500
Ruahapia Road (northern end)	10,500	-	-10,500
Elwood Road (southern end)	6,750	3,000	-3,750
Elwood Road (northern end)	5,500	4,000	-1,500
Otene Road	2,250	1,000	-1,250
Anderson Road	10,500	5,250	-5,250
Railway Road (through Whakatu)	3,000	2,000	-1,000
Pakowhai Road (north of Farndon Road)	9,000	10,750	+1,750
Pakowhai Road (south of Whakatu Arterial)	13,000	15,750	+2,750
Karamu Road (south of Whakatu Arterial)	18,750	17,500	-1,250
SH2 (north of Whakatu Arterial)	16,500	12,750	-3,750
SH2 (north of Clive)	16,250	15,500	-750
Napier Road (south of SH2)	4,000	6,000	+2,000
St Georges Road	5,500	3,750	-1,750

Table 6 Do Minimum and Project 2026 Annual Daily Traffic Flows

Note: Values rounded to nearest 250 vehicles.

The network effects are similar to that reported in the HPTS Project Feasibility Report. Flows in local areas are redistributed from the adjacent Elwood Road, Otene Road and Ruahapia Road routes.

A reduction in traffic flows through the Whakatu settlement on Anderson and Railway Roads together with a reduction of traffic using the SH2 coastal route are expected however more traffic will use Pakowhai Road to reach the Expressway. There will also be an increase in traffic along Napier Road from Havelock North however this is balanced by a reduction in traffic using the St Georges Road route to Havelock North.

Periodic traffic counts and surveys can be undertaken to determine the actual impact on the roading network.

Railway Level Crossing

An economic evaluation was undertaken of the effects of the level crossing on the operation of the arterial road in accordance with the NZTA's Economic Evaluation Manual. The level of rail traffic fluctuates however the calculations are based on a mean service of 9 trains a day with one passing in each peak hour as a worst case scenario.

The increase in journey times and increase in vehicle operating times for the percentage of vehicles held or delayed at the crossing was calculated. Using the EEM crash models the additional crash costs inherent with this type of level crossing were also determined. The net present value of these costs amounts to \$1.4M over a 30 year period.

However the removal of the Ruahapia Road crossing which has substandard horizontal and vertical alignment on both approaches will result in a reduction in crashes. This saving is taken into account in the economic evaluation.

6.3.2 Productive Land Use

A key driver through the EBD process was to reduce impacts on productive land, and route selection has taken this into consideration. However, the project will result in the loss of some land from productive use. It is noted that the remaining agricultural land alongside the new road is expected to remain in agricultural use, except for a small area where severance effects have made this unfeasible (refer Productive Land Use Impact Assessment, AgFirst 2014). The total area of land lost due to the road corridor and severance is approximately 20 ha.

There is potential reverse sensitivity effects associated with crop spraying and road users. The proposed conditions (HDC 2014d) include a requirement to provide shelterbelt planting or similar measures to address these effects, if requested by landowners.

The alignment runs through existing shelterbelts that will need to be partly removed for the road corridor. Any replacement or additional planting will need to be outside of the clear zone of the arterial road.

6.3.3 Noise

Key properties that will be impacted by noise are:

- The Haley property, where the WAL is within 15m of the residence;
- The Omahuri Orchards Property, where the WAL is within 60m of the residence;
- Pt Lot 1 DP 14513 Ward property, where the WAL passes within 30m of the residence; and
- The Dillion property, where the WAL is elevated and within 140m of the residence.

The cost estimate for the project has made some allowance for noise mitigation measures.

The Noise Assessment (Hegley 2014) identifies specific mitigation measures.

Construction noise is discussed in Section 5.10.3 of this report.

6.3.4 Vibration

The road will be in close proximity to industrial sites, some of which involve sensitive processing equipment. The effects of vibration from the operation of the WAL are investigated further in the Noise Assessment (Hegley 2014).

6.3.5 Landscape, Natural Character and Amenity

The road is proposed to pass through a relatively flat, rural area which is predominantly in horticultural and industrial use. The Karamu Stream is a prominent feature.

Approximately 4 dwellings are likely to have their visual amenity affected, with rural views impacted by the construction of a road corridor with inherent traffic volumes expected. There are however several other roads in this urban fringe environment and the road is not out of place in the environment.

The Landscape and Visual Assessment (Isthmus 2014) assesses these impacts.

6.3.6 Natural Hazards

Potential natural hazards affecting the proposed road include flooding and earthquakes.

The bridge and embankments are to be designed for seismic loading. Drainage is designed for storm events of 1 in 10 years for the swales and 1 in 20 years for the culverts. As with the Council's roading network special inspections will be required after any significant event to check the safety of the road.

Potential impacts from natural hazards are assessed in the Natural Hazards Assessment (GHD 2014d).

6.3.7 Ecology and Water Quality

Initial assessments indicate that whilst the Clive River has relatively good water quality the Karamu Stream that feeds into it has a very poor water quality.

The Stormwater Management Plan and Erosion and Sediment Control Plan identify approaches to managing impacts on water quality. Section 4.12 identifies bridge design elements and construction approaches.

The Aquatic and Terrestrial Ecology Report (EAM 2014a) assesses these impacts in consideration of the proposed approach to stormwater management.

6.3.8 Cultural Heritage and Archaeology

A preliminary assessment has been undertaken to identify Waahi Tapu, Heritage and Archaeological sites within or near the project area.

A key factor identified early in the project development and further reinforced during the EBD was to avoid Maori Land and historical sites from the final design option. The preliminary assessment identified Waahi Tapu sites and culturally significant areas based on information reported in the District Plan

Waahi Tapu Sites

Nine Waahi Tapu sites were identified within or near the project area based on information provided in the Hastings District Plan. These are summarised in the table and figures below:

Table 7 Waahi Tapu Sites

Ref.	No.	Plot Description	Туре	Location
W12	34	Pt Pakowhai Maori Reserve 4 Blk XIII Heretaunga SD	Urupa	Pakowhai/Whakatu
W13	33, 34	Pakowhai 5CI, 5B2, 5A1 Blk Heretaunga SD Lot 1 DP 17171 Blk XII Heretaunga SD Sec 1 SO9684 BLK XII Heretaunga SD Res 28 Pakowhai 5A2 5A3 Blk XII Heretaunga SD Park Pakowhai Maori Reserve 1; Part Pakowhai Maori Reserve2; Sec 2, Sec2 and Sec 3 SO 10742; Part Pakowhai Maori Reserve 3; Pt Lot 1 DP 5302: Pakowhai 5A2 Sec 1 SO 9684 Pakowhai 5B3 Pakowhai Maori Reserve 5C	Battlefield - Urupa	Pakowhai/Whakatu
W14	41	Pt Mangateretere West Blk X Blk XII Heretaunga SD Lots 1 3 DP 13557 Lot 3 15 Esplanade Res Blk XII Heretaunga Lot 2 DP 12384 Lot 2 DP 13557 3 Blk XII Heretaunga SD	Battlefield - Urupa	Pakowhai/Whakatu
W32	41	Lot 2 DP 22494 Blk XVI	Old Pa Sites, Urupa, Pa Site	Ruahapia
W33	41	Karamu AYIB, AYIA, AY2 Blk XVI Heretaunga SD	Tauranga Waka	Ruahapia
W34	41	Pt Karamu AY2 Blk XVI Heretaunga SD	Battlefields – Taonoke	Ruahapia
W36	48	Karamu C2C2B5 Blk XVI Heretaunga SD	Tauranga Waka – Mahinga Kai	Waipatu
W37	48	Karamu C2C2B5 Blk XVI Heretaunga SD	Urupa - Pareranui	Waipatu
W38	48	Lot 1 DP 22675	Urupa	Waipatu
W39	48	Pt Lot 2 DP 11378 Pt Sec 21 Blk XVI	Karamu Pa Site	Waipatu

Figure 12 Location of Waahi Tapu Sites (1)



Note: W38 etc relates to Table 7 Waahi Tapu sites

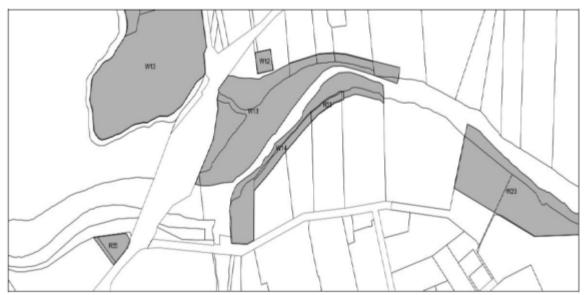


Figure 13 Location of Waahi Tapu Sites (2)

Note: W12 etc relates to Table 7 Waahi Tapu sites. R55 relates to a Council Reserve

None of the identified sites are located within the road corridor.

The Cultural Impact Assessment (Ipurangi Developments 2014) provides further analysis.

Archaeological Sites

The alignment of the WAL will avoid all known archaeological sites in the area. A preliminary assessment identified two sites in the vicinity of Ruahapia Road and SH2 and three sites to the west in the vicinity of Rangitane, Pakowhai and Farndon Roads. These sites are all outside of the proposed designation.

The Archaeological Assessment (Clough 2014) provides further analysis.

Heritage Trees

No Historical trees were identified on the project route. A preliminary assessment identified listed heritage trees on Otene Road however these are well outside the project area.

6.3.9 Social-cultural effects

The Enquiry by Design Process involving the community was used to determine the preferred alignment of the road to ensure that social and cultural effects are taken into account in the earliest stages of design. The route is away from residential areas however passes close to some residents near the State Highway 2 intersection.

During construction there will be an increase in truck movements however these are not expected to be through the Whakatu residential area. There may be some increase in heavy traffic along the State Highway roading network and Pakowhai Road during construction but this is not expected to result in any significant worsening of existing traffic conditions for residents during the construction period.

The Social Impact Assessment (GHD 2014e) considers these impacts in more detail.

7. Basis of Report

This report has been prepared by GHD for Hastings District Council and may only be used and relied on by Hastings District Council for the purpose agreed between GHD and the Hastings District Council as set out in Section 1 of this report.

GHD otherwise disclaims responsibility to any person other than Hastings District Council (and GHD's wider team of sub-consultants undertaken assessments on the basis of this report) arising in connection with this report. GHD also excludes implied warranties and conditions, to the extent legally permissible.

The services undertaken by GHD in connection with preparing this report were limited to those specifically detailed in the report and are subject to the scope limitations set out in the report.

The opinions, conclusions and any recommendations in this report are based on conditions encountered and information reviewed at the date of preparation of the report. GHD has no responsibility or obligation to update this report to account for events or changes occurring subsequent to the date that the report was issued.

The opinions, conclusions and any recommendations in this report are based on assumptions made by GHD described in this report. GHD disclaims liability arising from any of the assumptions being incorrect.

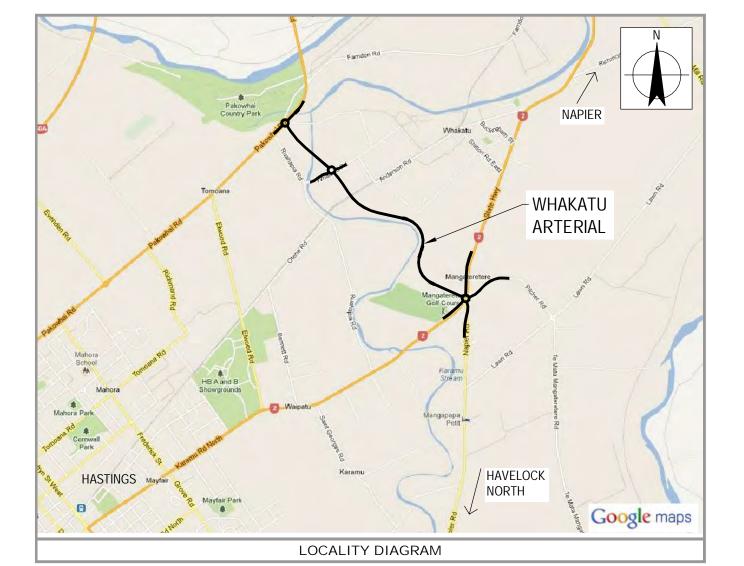
Appendices

Appendix A - Technical Drawings

51-31468

















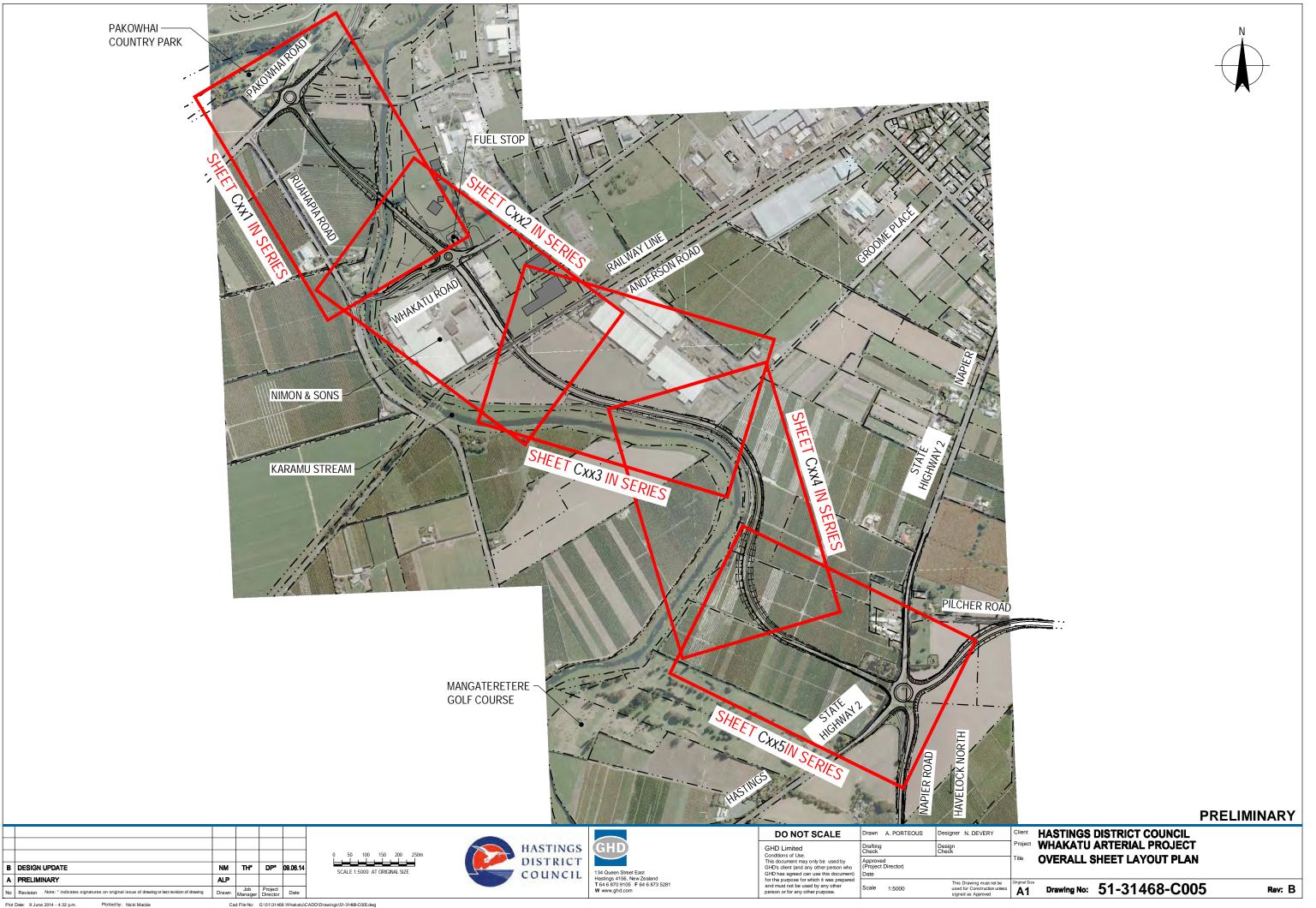
DRAWING REGISTER AND TRANSMITTAL NOTICE ISSUE

RAV	VING REGISTER AND TRANSMITTAL	NOTIO	CE				SS	UE			
		DAY MONTH		24 10	04 11		04 12	12 12	09 06		Γ
No.	DRAWING TITLE	YEAR			13	11 13	12	12	14		
G001	COVER SHEET		А	_	_	-	-	в	С		
G002	DRAWING REGISTER & TRANSMITTAL NOTICE		A	-	-	-	-	В	С		
C005	OVERALL SHEET LAYOUT PLAN		А	-	-	-	-	-	В		
C101	SITE SURVEY PLAN - SHEET 1		А	-	-	-	-	В	С		
C102	SITE SURVEY PLAN - SHEET 2		А	-	-	-	-	-	В		
C103	SITE SURVEY PLAN - SHEET 3		А	-	-	-	-	-	В		
C104	SITE SURVEY PLAN - SHEET 4		А	-	-	-	-	-	В		
2105	SITE SURVEY PLAN - SHEET 5		А	-	-	-	-	-	В		
2201	LAYOUT PLAN AND LONGITUDINAL SECTION - SHEET 1		А	-	-	-	-	В	С		
202	LAYOUT PLAN AND LONGITUDINAL SECTION - SHEET 2		A	-	-	-	-	-	В		
203	LAYOUT PLAN AND LONGITUDINAL SECTION - SHEET 3		А	-	-	-	-	-	В		
204	LAYOUT PLAN AND LONGITUDINAL SECTION - SHEET 4		А	-	-	-	-	-	В		
205	LAYOUT PLAN AND LONGITUDINAL SECTION - SHEET 5		А	-	-	-	-	-	В		
2211	PAKOWHAI ROAD INTERSECTION LAYOUT		А		_	_	_	В	с		
212	WHAKATU ROAD INTERSECTION LAYOUT		A	-	_	-	-	В	c		
213	ANDERSON ROAD A INTERSECTION LAYOUT		A	_	_	-	-	В	С		
214	STATE HIGHWAY 2 / NAPIER ROAD & PILCHER ROAD INT. LAY	OUT	A	-	-	-	-	-	В		
215	LONGITUDINAL SECTION ROADS 02, 03, 04		^				-	-	В		
215	LONGITUDINAL SECTION ROADS 02, 03, 04		A A	-	-	-	-	-	В		
217	LONGITUDINAL SECTION ROAD 00 & 07		A	-	_	-	-	-	B		
221	ROAD 01 - TYPICAL CROSS SECTIONS AND PAVEMENT DESIG	N	А	-	-	-	-	-	В		
222	ROAD 01 - TYPICAL CROSS SECTIONS AND PAVEMENT DESIG	N		-	-	-	-	-	В		
231	CONTOUR PLAN - SHEET 1		А	_	-	-	-	В	С		
232	CONTOUR PLAN - SHEET 2		A	-	_	-	_	-	В		
233	CONTOUR PLAN - SHEET 3		А	-	-	-	-	-	В		
234	CONTOUR PLAN - SHEET 4		А	-	-	-	-	-	В		
235	CONTOUR PLAN - SHEET 5		А	-	-	-	-	-	В		
241	ROADMARKING PLAN - SHEET 1		А		-		-	-	в		
242	ROADMARKING PLAN - SHEET 2		A	-	_	-	-	-	В		
243	ROADMARKING PLAN - SHEET 3		А	-	-	-	-	-	В		
244	ROADMARKING PLAN - SHEET 4		А	-	-	-	-	-	В		
245	ROADMARKING PLAN - SHEET 5		А	-	-	-	-	-	В		

DRA	WING REGISTER AND TRANSMITTAL						SS				
No.	DRAWING TITLE	DAY MONTH YEAR	09	10	11	11	04 12 13	12	14 01 14	09 06 14	
C251 C252 C253 C254 C255 C257 C257 C259 C260 C261	CROSS SECTIONS - ROAD 01- CH 50 TO CH 400 CROSS SECTIONS - ROAD 01- CH 725 TO CH 1085 CROSS SECTIONS - ROAD 01- CH 1165 TO CH 1500 CROSS SECTIONS - ROAD 01- CH 1550 TO CH 1900 CROSS SECTIONS - ROAD 01- CH 2350 TO CH 2300 CROSS SECTIONS - ROAD 01- CH 2750 TO CH 2700 CROSS SECTIONS - ROAD 10- CH 50 TO CH 350 CROSS SECTIONS - ROAD 11- CH 50 TO CH 350 CROSS SECTIONS - ROAD 11- CH 450 TO CH 780									B B B B B B B B B B B B B B B B B B B	
DIS	TRIBUTION		FO	RN	/IA1	Г/С	OP	PIES	S		
	NGS DISTRICT COUNCIL PRELIMINARY DESIGN		P=PC)F, H=H	HARD(JUPY				1	
THO TH	WGS DISTRICT COUNCIL PRELIMINARY DESIGN									1	

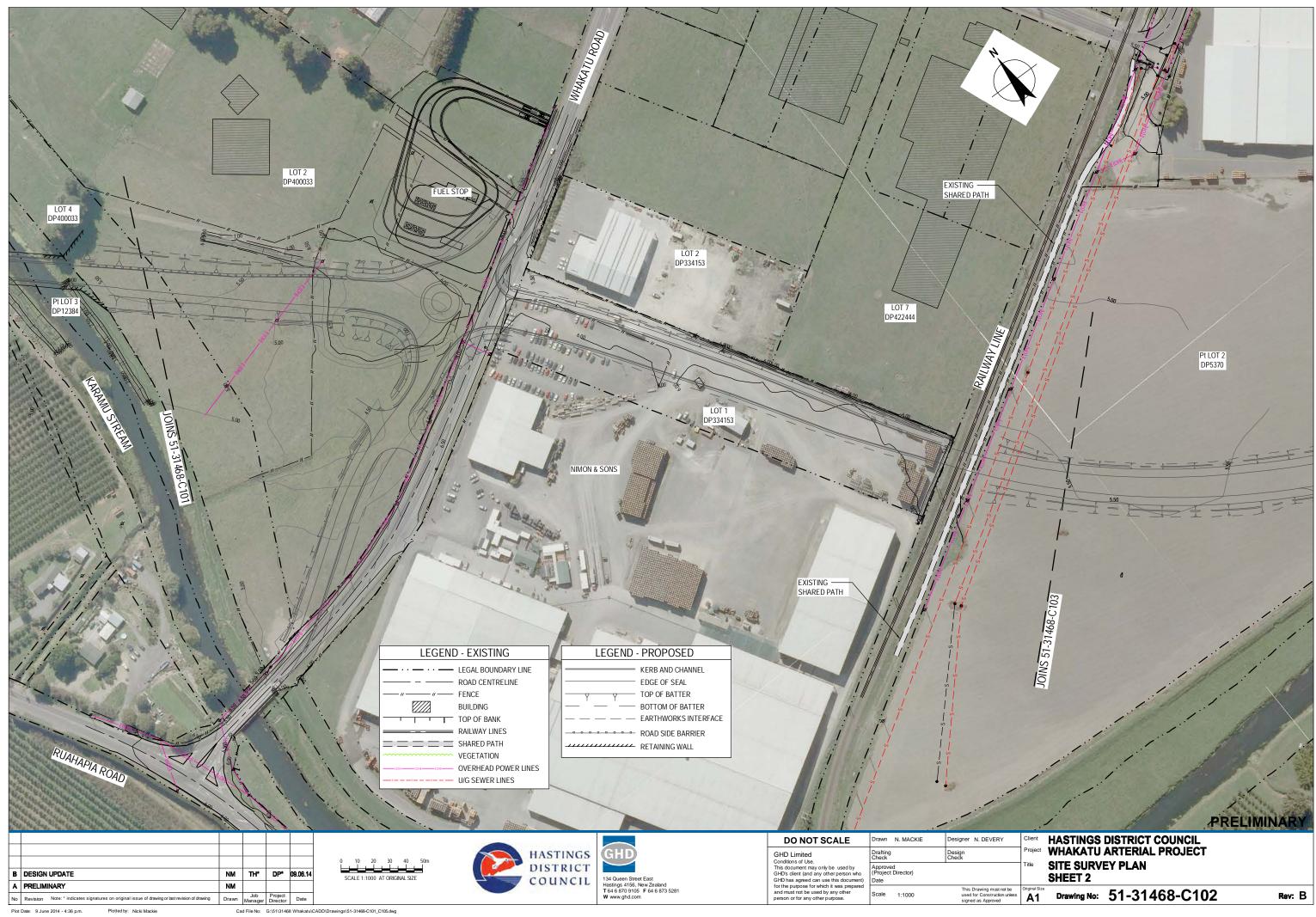
						DO NOT SCALE	Drawn N. MACKIE	Designer N. DEVERY		HASTINGS DISTRICT COUNCIL
C DESIGN UPDATE	NM TH*	0.0*	09.06.14	HASTINGS		GHD Limited Conditions of Use.	Drafting Check	Design Check		WHAKATU ARTERIAL PROJECT
B BERM & LAND REQUIREMENTS UPDATED	NM III		03.00.14	DISTRICT		This document may only be used by GHD's client (and any other person who GHD has agreed can use this document)	Approved (Project Director)		Title	DRAWING REGISTER & TRANSMITTAL NOTICE
A PRELIMINARY No Revision Note: * indicates signatures on original issue of drawing or last revision of drawing	NM Job	Project	t. Date	COUNCIL	Hastings 4156, New Zealand T 64 6 870 9105 F 64 6 873 5281	for the purpose for which it was prepared and must not be used by any other person or for any other purpose.	Scale NOT TO SCALE	This Drawing must not be used for Construction unless	Original Size	Drawing No: 51-31468-G002 Rev: C
Plot Date: 10 June 2014 - 10:02 AM Plotted by: Nigel Devery	Cad File No:	G:\51\314		tu/CADD/Drawings/51-31468-G001_G002.dwg	-			signed as Approved		

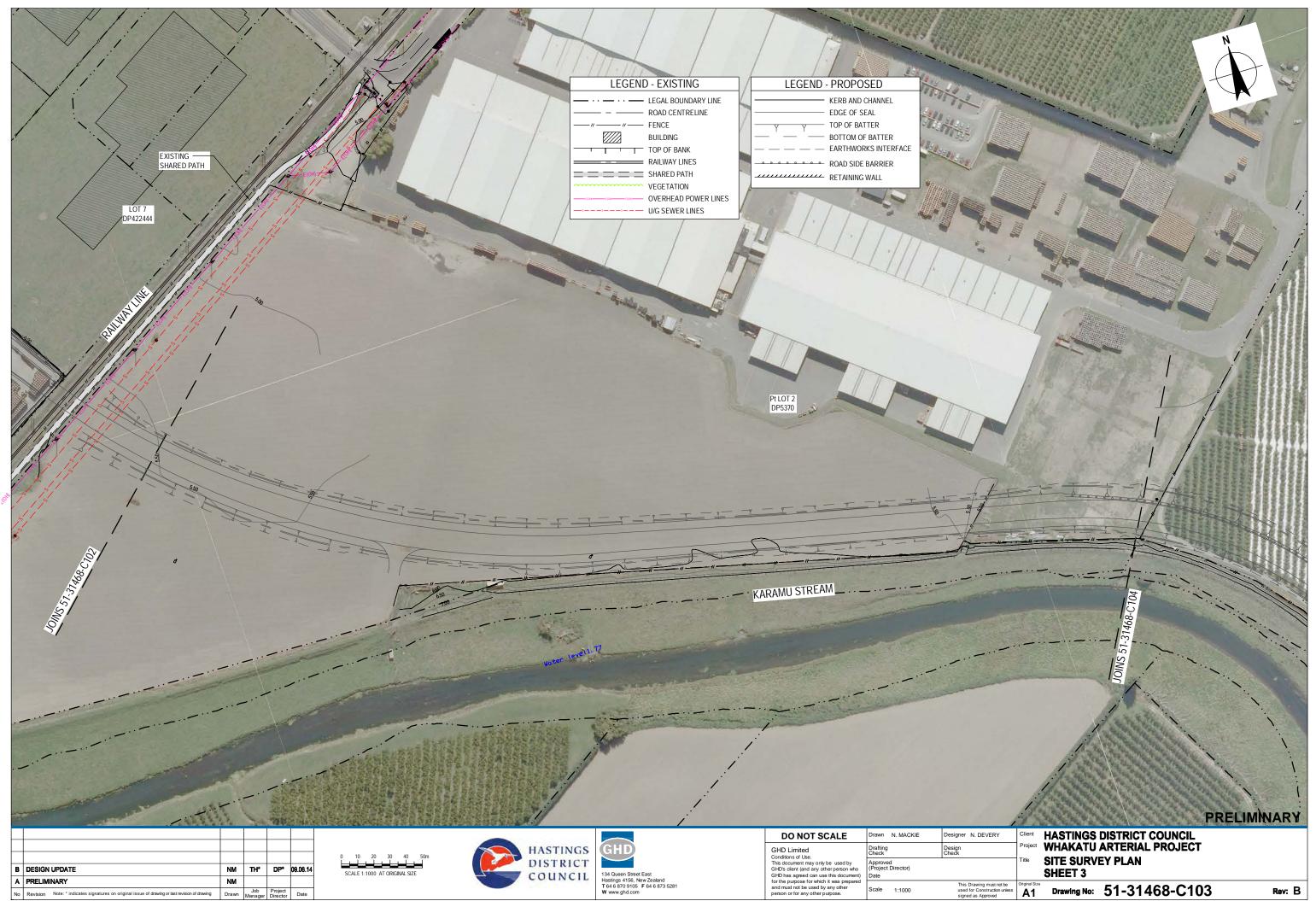
PRELIMINARY

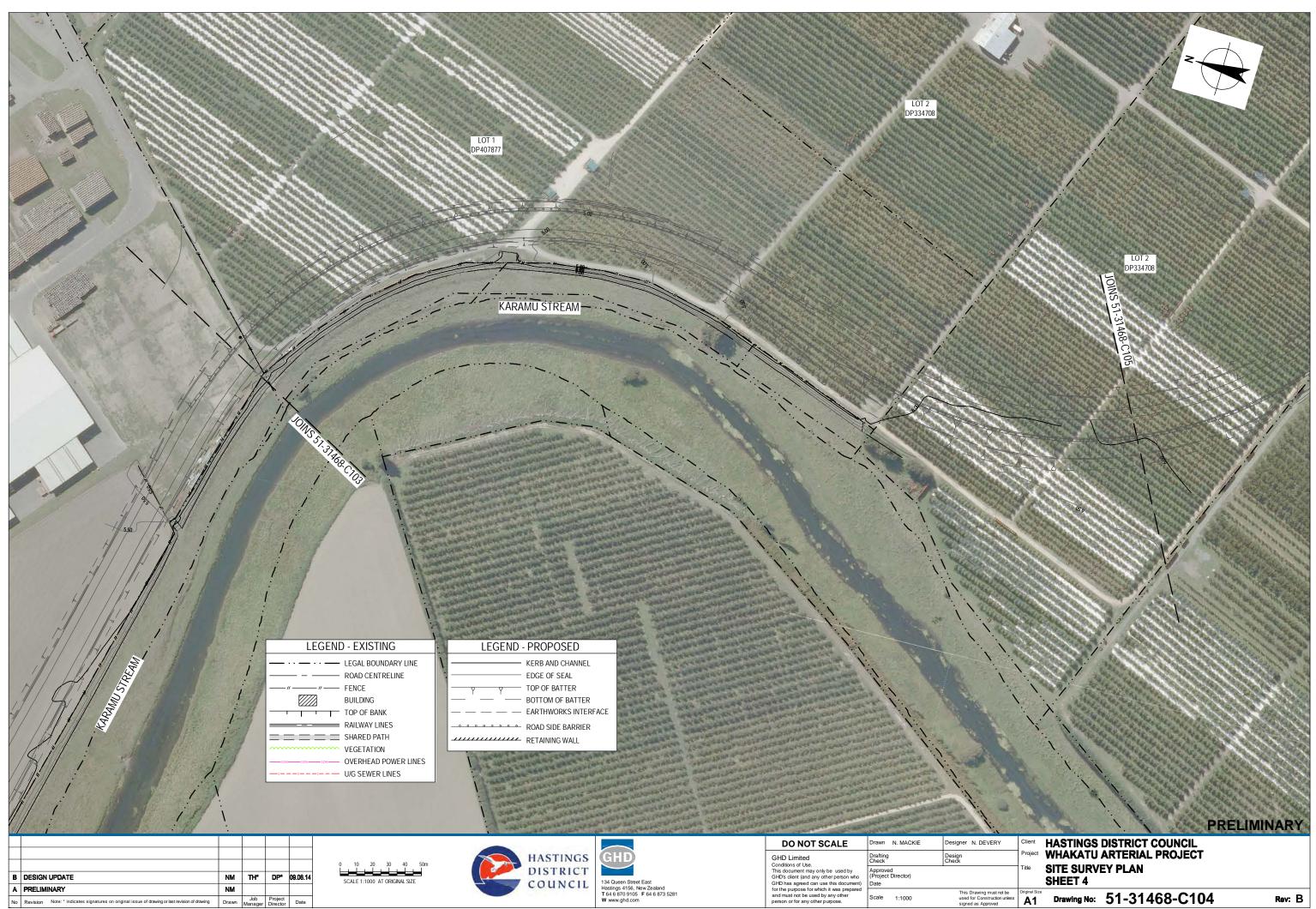


Plot Date: 9 June 2014 - 4:32 p.m. Plotted by: Nicki Mackie



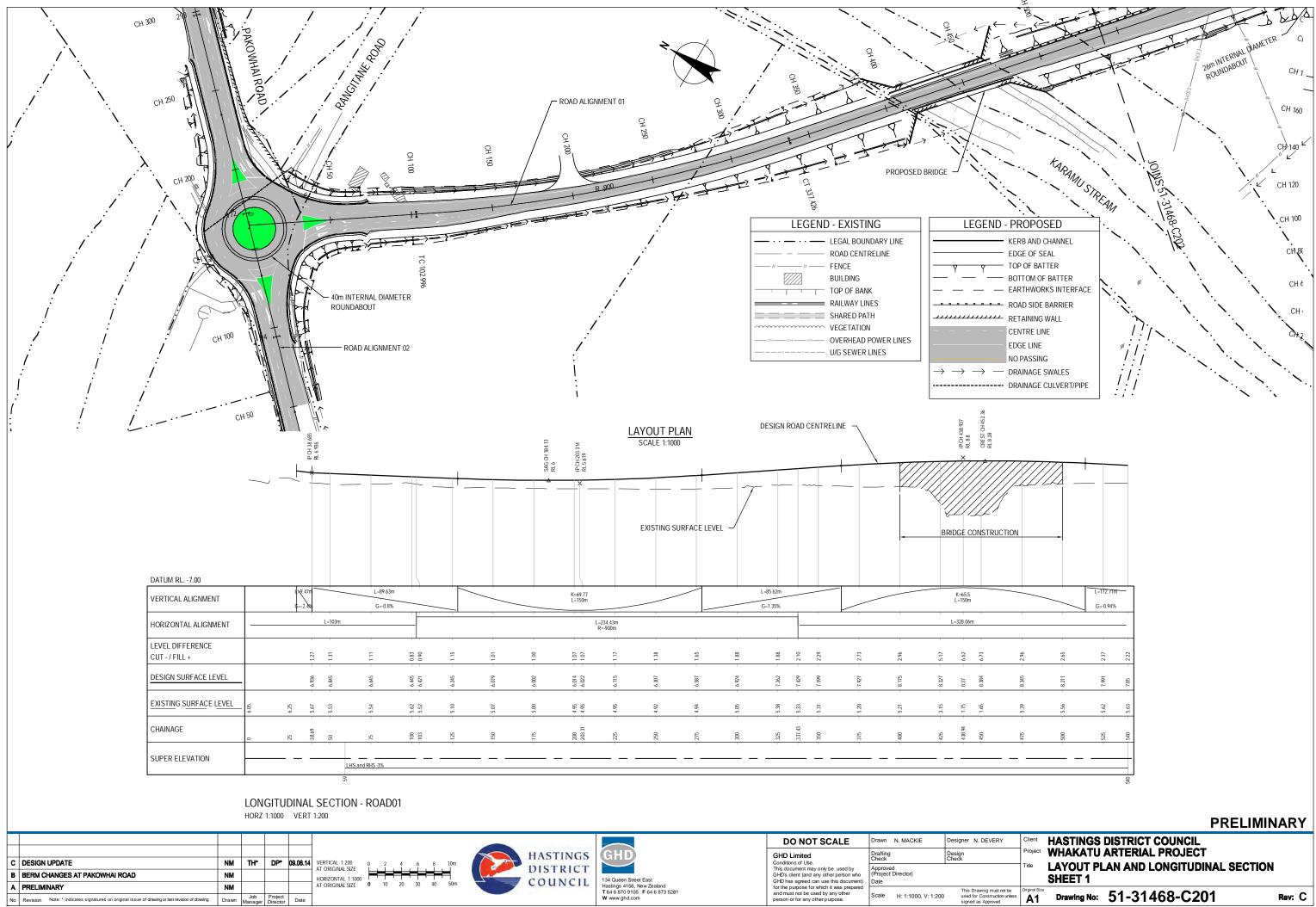






Cad File No: G:\51\31468 Whakatu\CADD\Drawings\51-31468-C101_C105.dwg





	Client Project		DISTRICT COUNCIL ARTERIAL PROJECT	
	Title	LAYOUT PL SHEET 1	AN AND LONGITUDINAL SEC	TION
		SHEELL		
be Inless	Original Size	Drawing No:	51-31468-C201	Rev: C

				с _{н 850}	LIGNMENT 04	ROAD A RETAIN WALL	CH DE HO			250	CH/650 F		CHF600			γ	
	CH 1000	1000 1100 1100 1100 1100 1100 1100 110			CT &	······································			To a start of the					A A A A A A A A A A A A A A A A A A A		JOINS 51-31468-C2	KARAMU STREAM
DATUM RL 300 UNIX MUX DUM NUM DABUIT EXISTING SUBFACE LEVEL 0		202 	RI 7052	AN .					R 6432	1/13.862 1992			CH 100 CH 80 CH 80) 	
DATUM RL - 3.00 VERTICAL ALICAMENT LORDINAL SECTION - ROADDI LUNCH RL - 3.00 LUNCH RL -	033.001			CREST CH 911		AD CENTRELIN	DESIGN RO	P CH 733.028 RI	P CH 709.835 RI	IP CH 686.043 Rt							
VERTICAL ALIGNMENT L-112/10 L-122/00 L-123/00 L-123/00 L-105/00 HORIZONTAL ALIGNMENT L-200/00 L-200/00 L-105/00 L-105/00 L-105/00 LEVEL DIFFERENCE L-105/00 L-105/00 L-105/00 L-105/00 L-105/00 DESIGN SURFACE LEVEL 9 <th></th> <th></th> <th></th> <th>ELEVEL</th> <th>EXISTING SURFACE</th> <th></th> <th></th> <th></th> <th>NDABOU</th> <th>I- ROUN</th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th>				ELEVEL	EXISTING SURFACE				NDABOU	I- ROUN							
C-0.9% C-0.3% C-0.3% HORIZONTAL ALIGNMENT L-328.00m R-4500 R-4000 LEVEL DIFFERENCE L-146.35m L-146.35m R-4000 CUT -/ FILL + FC -0.7% L-146.35m R-4000 DESIGN SURFACE LEVEL 90 <t< td=""><td></td><td></td><td>K=150.02 L=120m</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>			K=150.02 L=120m														
CUT - I FILL + TOT FIL	<u> </u>	G=-0.5%			L=74.67m	G=0.3%		L=146.53m R=600m	1				1	1	······		
DESIGN SURFACE LEVEL 0	0.67 - 0.17 - 0.18 - 0.18 - 0.18	0.75 - 0.73 - 0.75 - 0.7	0.89 - 0.89 - 0.89 - 0.82 - 0.8		- 0.86	0.73		0.45 - 0.27 -	0.14	0.03	0.07	2.19	- 1:75	1.92	5.03	2.22 - 2.13	
EXISTING SURFACE LEVEL 99 <	1 1 1			1	1	1 1	1	1 1	1		1 1	1	1	1	1	1 1	
CHAINAGE 0 1 <th1< th=""> 1 <th1< th=""> 1 <th1< th=""> <th1< td="" th1<=""><td>1 1 1</td><td>1 1 1</td><td></td><td>1</td><td>1 1</td><td>1 1</td><td>1</td><td></td><td>1</td><td></td><td>1 1</td><td>I</td><td></td><td>1</td><td>49</td><td>1 1</td><td>EXISTING SURFACE LEVEL</td></th1<></th1<></th1<></th1<>	1 1 1	1 1 1		1	1 1	1 1	1		1		1 1	I		1	49	1 1	EXISTING SURFACE LEVEL
SUPER ELEVATION	1 1 1			1 1	1	1 1	1	1 1	1		1 1	1	1	I	I	1 1	CHAINAGE
LONGITUDINAL SECTION - ROAD01	10.100		- 92 - 95 - 95 - 95 - 977	- 87	- 82 - 85	81;			- 10	69 - 100	1 1		- 62!				SUPER ELEVATION
	1023							742								540	L
												l	- Roado				

HASTINGS DISTRICT

COUNCIL

GHD

134 Queen Street East Hastings 4156, New Zealand T 64 6 870 9105 F 64 6 873 5281 W www.ghd.com GHD Limited Conditions of Use. This document may only be used by GHDS cieffic (and any other person who GHD has agreed can use this document) for the purpose for which it was prepared and must not be used by any other person or for any other purpose.

Drafting Check

Date

Approved (Project Director)

Scale H: 1:1000, V: 1:200

Design Check

 No
 Revision
 Note: * indicates signatures on original issue of drawing or last revision of drawing
 Drawn
 Job Manager
 Project
 Date

 Plot Date:
 9 June 2014 - 4:45 p.m.
 Plotted by: Nicki Mackie
 C ad File No:
 G:\$15131468 WhakatuiCADD\Drawings\51-31465C201_C205.dwg

VERTICAL 1:200 AT ORIGINAL SIZE

10 20 30

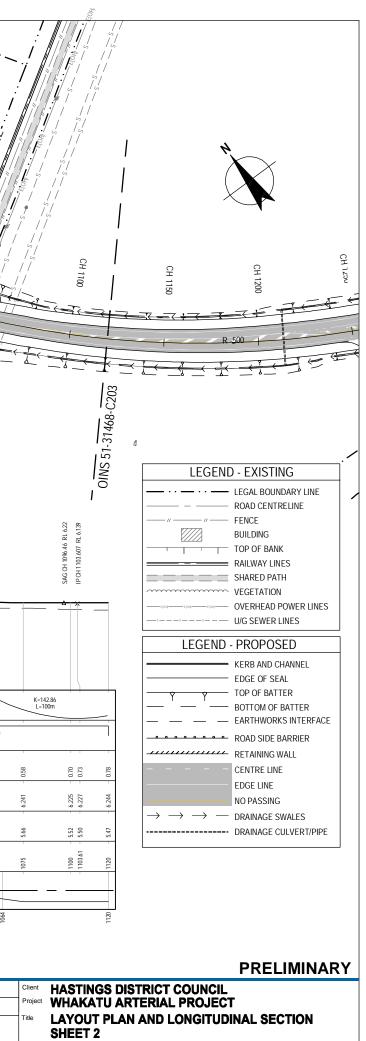
09.06.14 HORIZONTAL 1:1000 AT ORIGINAL SIZE 0

NM TH*

NM

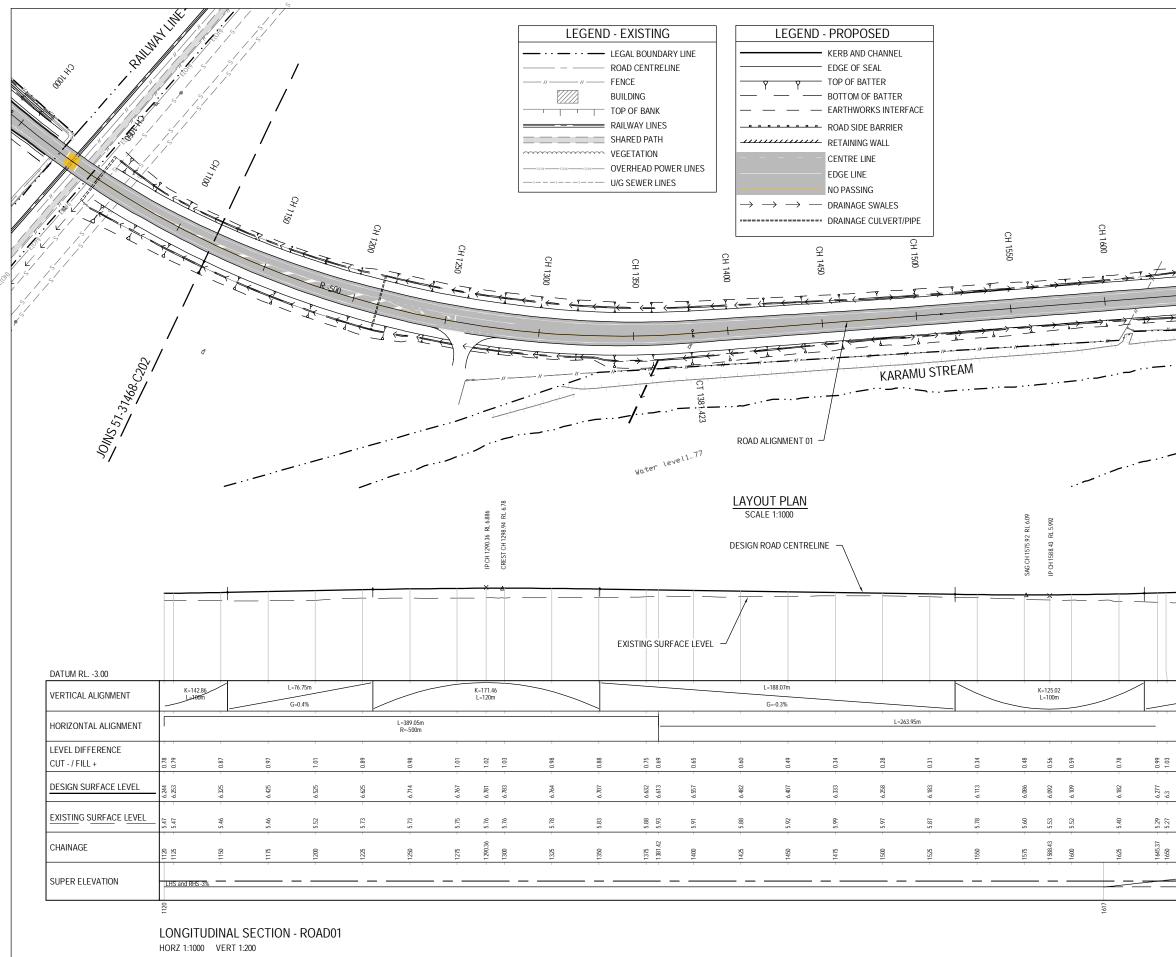
B DESIGN UPDATE

A PRELIMINARY



This Drawing must not be used for Construction unless A1 Drawing No: 51-31468-C202

Rev: B



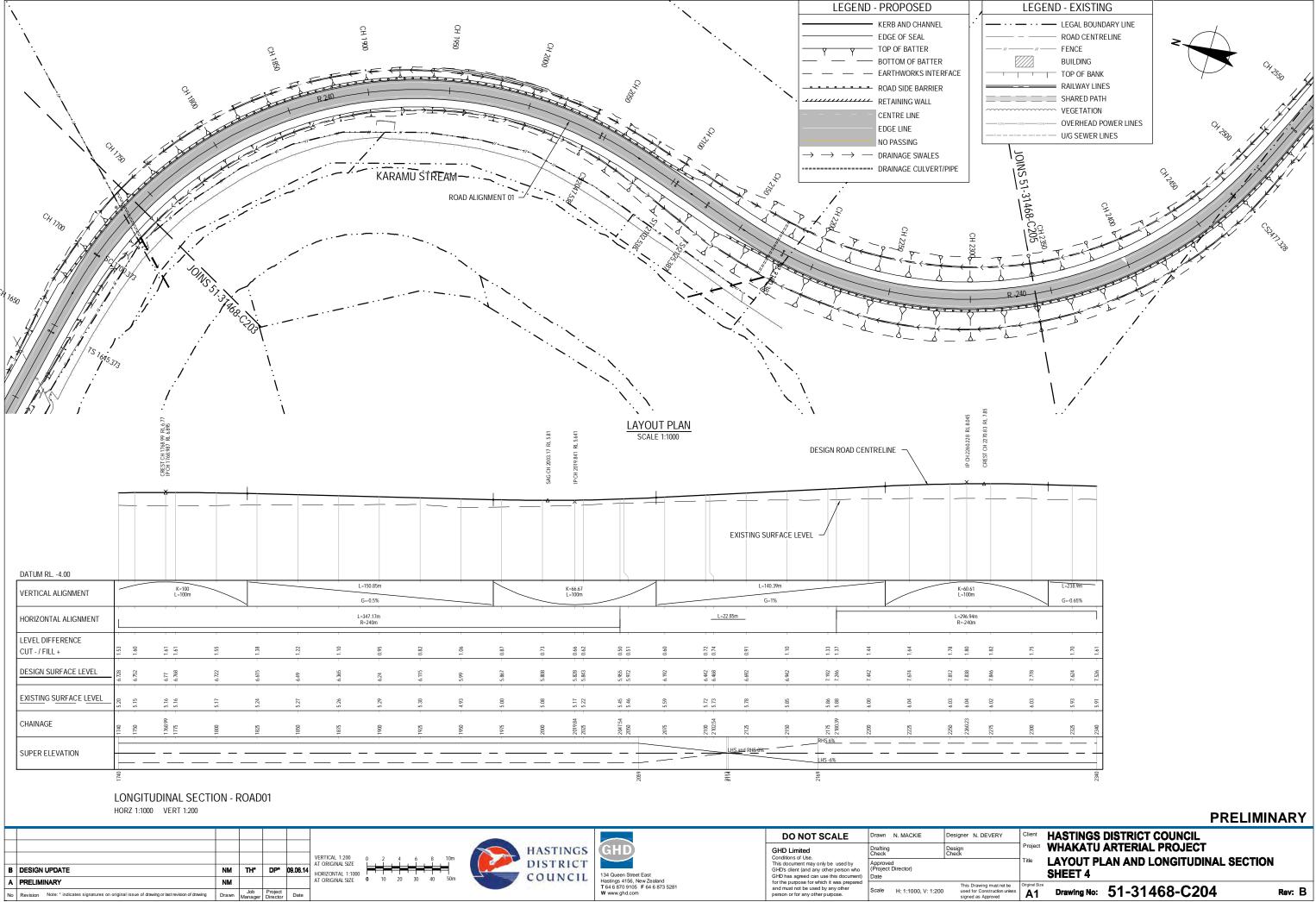
					_	HAS	INGS GHD	DO NOT SCALE GHD Limited		Designer N. DEVERY Design Check
E	DESIGN UPDATE	NM	TH* D	P* 09.06.'	HURIZUNTAL I:1000	DIS1	RICT NCIL 134 Queen Street East Hastings 4156 New Zealand	Conditions of Use. This document may only be used by GHD's client (and any other person who GHD has agreed can use this document)	Approved (Project Director)	Спеск
		NM Drawn	Job Pro Manager Dire	ect ctor Date	AT ORIGINAL SIZE 0 10 20 30 40 50m	Cou	Hastings 4156, New Zealand T 64 6 870 9105 F 64 6 873 5281 W www.ghd.com	for the purpose for which it was prepared and must not be used by any other person or for any other purpose.		This Drawing must not be used for Construction unler signed as Approved

CH 1650	CH 1700
1 165	CH CH
50	1700 CH 1750
	8
	- Accordent to the B
	States and a state of the state
TS 1645 373	
TS 1695 373	

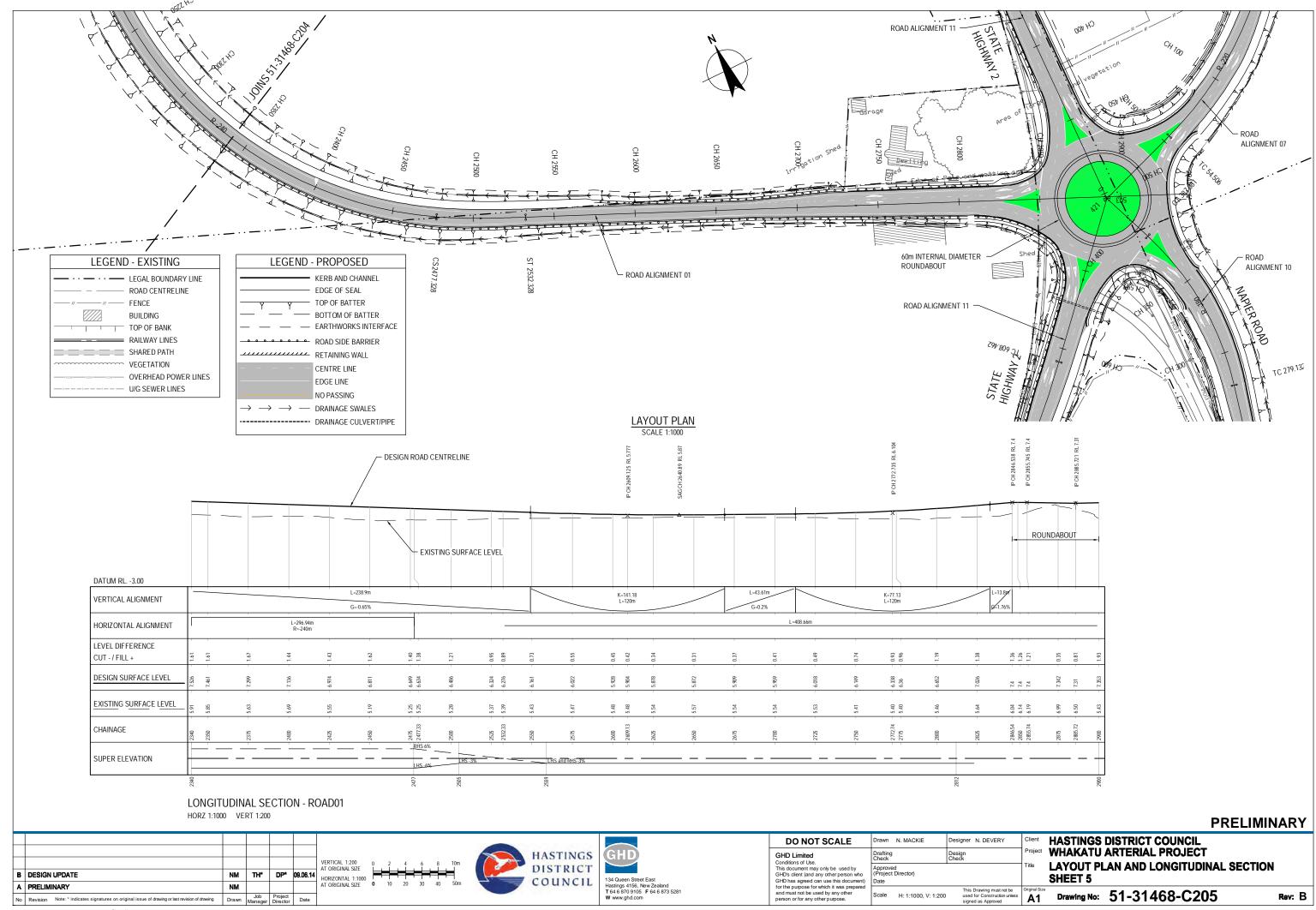
				-
			<u> </u>	
L=8	30.56m		K=100	
			L=100m	
G	=0.5%		\sim	
		1-3/	7.17m	
		R=2	240m	
			-	_
1.20	5	2	1.41	1.53
			-	-
5		-	2	8
6.425		0.0	6.673	6.728
				1
5.22	2	<u>•</u>	5.27	5.20
2	ŭ	ń	3	3
1675	Ę	3	1725	1740
	;	LHS 6%	-	
		L113 076		-
	_	DUC 49/		-
		RHS <u>-6%</u>		-
		1 /01		1740
		-		-

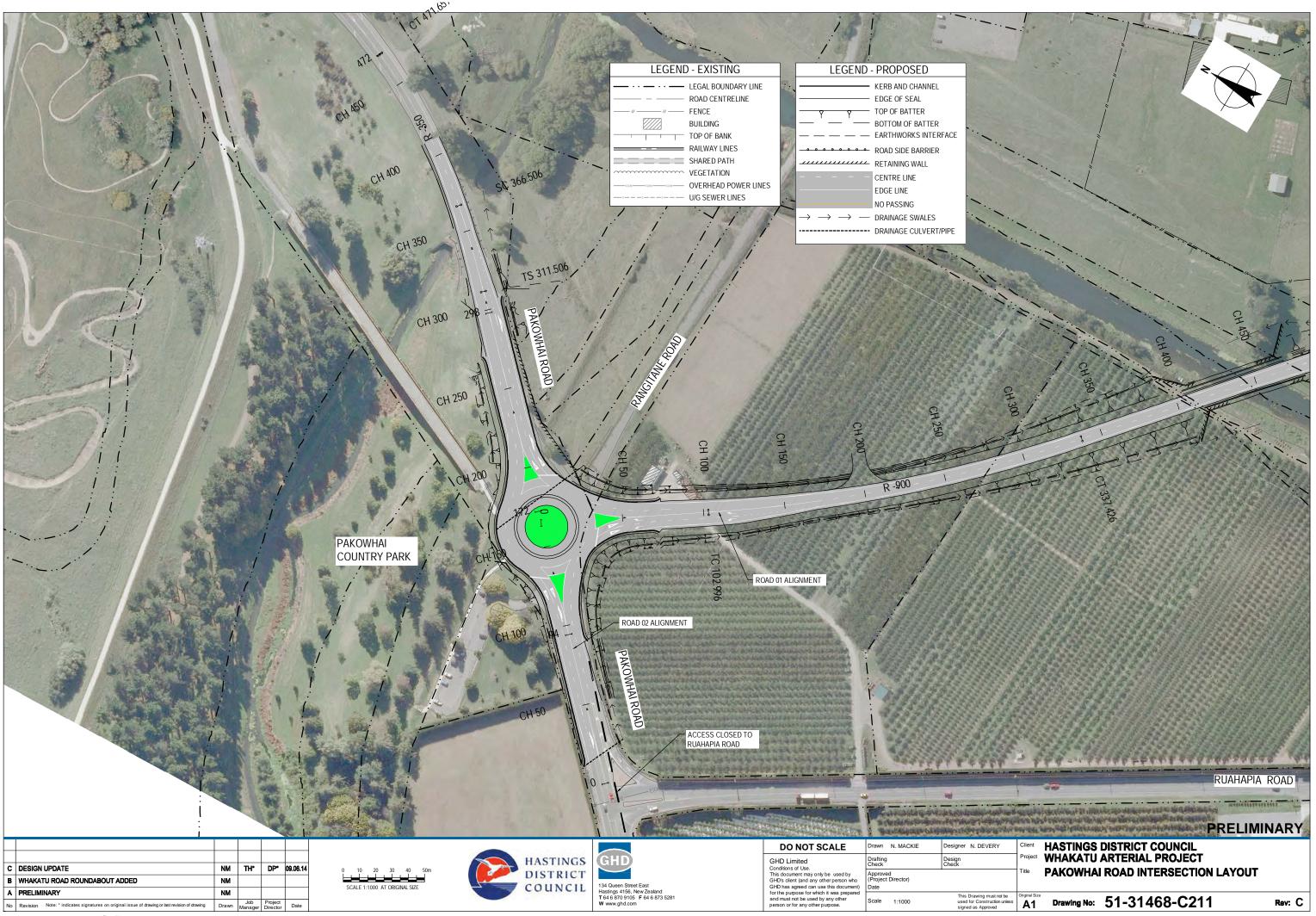
PRELIMINARY

	Client		DISTRICT COUNCIL		
	Project				
	Title	LAYOUT PL SHEET 3	AN AND LONGITUDINAL SECTION		
be unless	Original Size	Drawing No:	51-31468-C203	Rev:	В



	Client	HASTINGS	DISTRICT COUNCIL	
	Project	WHAKATU /	ARTERIAL PROJECT	
	Title	LAYOUT PL	AN AND LONGITUDINAL SECTION	
		SHEET 4		
ot be i unless	Original Size	Drawing No:	51-31468-C204	Rev: B





Plot Date: 9 June 2014 - 4:29 p.m. Plotted by: Nicki Mackie

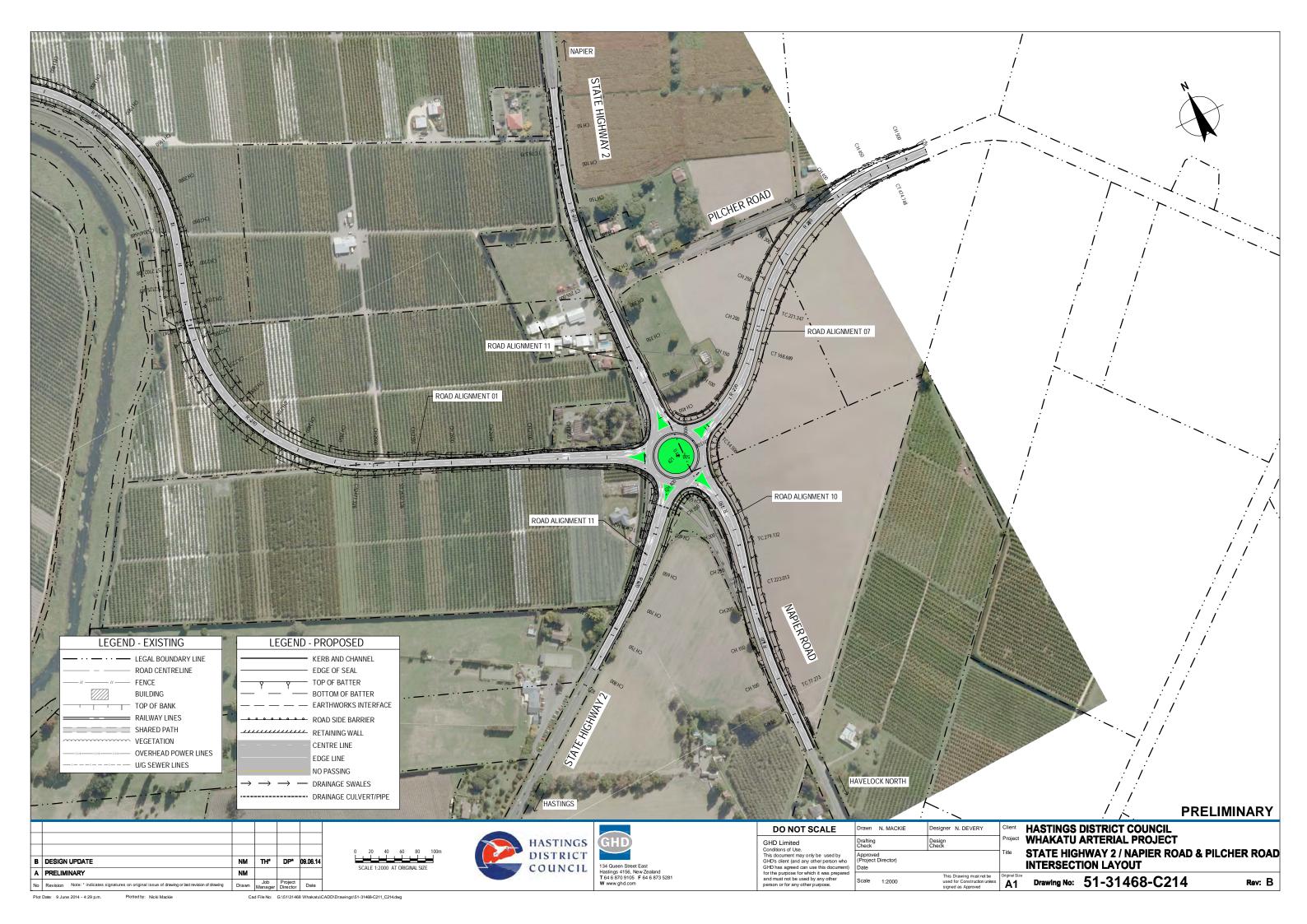
Cad File No: G:\51\31468 Whakatu\CADD\Drawings\51-31468-C211_C214.dwg

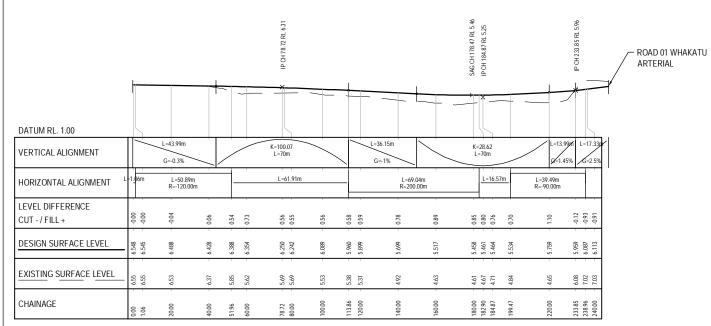
Image: Constraint of the purpose of ordinary other revision of drawing or last revision or drawing or last revi	LEGEND - EXISTING ROAD CENTRELINE FENCE BUILDING SHARED PATH VECETATION OVERHEAD POWER LINES UG SEWER LINES CH CH CH CH CH CH CH CH CH CH	LEGEND - PROPOSED KERB AND CHANNEL EDGE OF SEAL P P TOP OF BATTER BOTTOM OF BATTER ACTION OF BATTER ROAD SIDE BARRIER RETAINING WALL CENTRE LINE EDGE LINE NO PASSING DRAINAGE SWALES DRAINAGE CULVERT/PH	PE ROAD ALIGNMENT OI	CH 160		ROAD ROAD	T46.751 ALIGNMENT 04	Chan Martin Chan
	B WHAKATU ROAD ROUNDABOUT ADDED A PRELIMINARY	NM SCAL	0 20 30 40 50m = 1:1000 AT ORIGINAL SIZE	DISTRICT COUNCIL 134 Queen Street Hastings 4156, N 64 6 870 9105	lew Zealand F 64 6 873 5281	GHD Limited Conditions of Use. This document may only be used by GHD's dient (and any other person who GHD has agreed can use this document) for the purpose for which it was prepared and must not be used by any other	Drafting D Check C Approved (Project Director) Date	Design Check This Drawing must on the

Cad File No: G:\51\31468 Whakatu\CADD\Drawings\51-31468-C211_C214.dwg

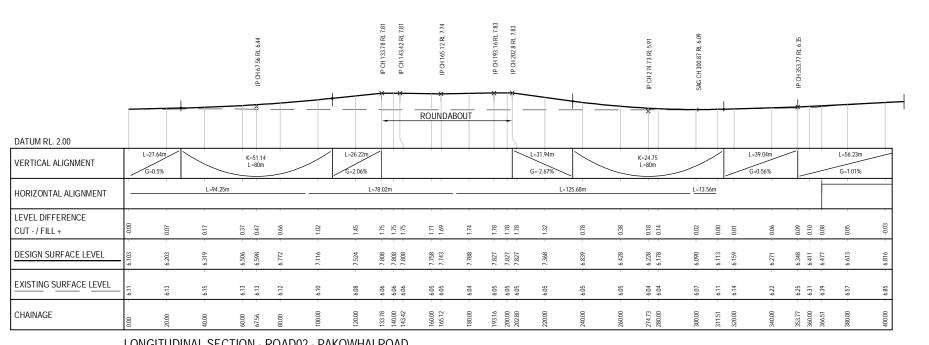








LONGITUDINAL SECTION - ROAD03 - WHAKATU ROAD SOUTH HORZ 1:1000 VERT 1:200



ROAD 01 WHAKATU ARTERIAL

VERTICAL ALIGNMENT
HORIZONTAL ALIGNMENT
LEVEL DIFFERENCE
CUT - / FILL +
DESIGN SURFACE LEVEL
EXISTING SURFACE LEVEL
CHAINAGE

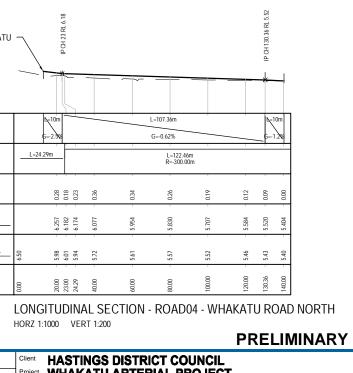
DATUM RL. 2.00

LONGITUDINAL SECTION - ROAD02 - PAKOWHAI ROAD

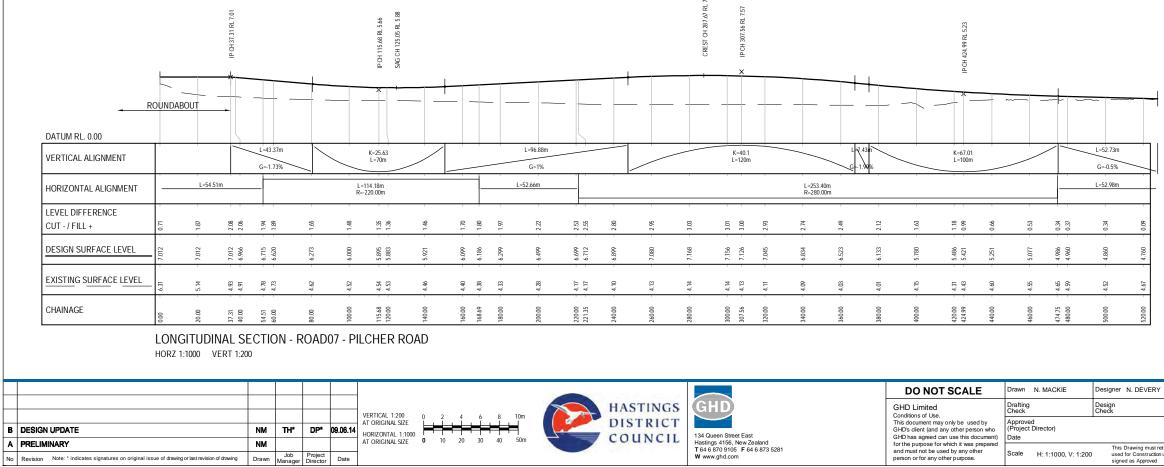
HORZ 1:1000 VERT 1:200

VERTICAL 1.200 0 2 4 6 8 10m HASTINGS GHD GHD Conditions of Use.	d Drafting Check	Design Check	Project WHAKATU ARTERIAL PROJECT
B DESIGN UPDATE NM TH* DP* 09.06.14 HORIZONTAL 1:1000 - 10 - 10 - 10 - 10 - 10 - 10 -	may only be used by and any other person who d can use this document) Approved (Project Dir Date	irector)	TINE SIDE ROAD LONGITUDINAL SECTIONS ROAD 02, 03 & 04
A PRELIMINARY NM A A Orthogram Output A Output A	e used by any other Scale H	H: 1:1000, V: 1:200 This Drawing must not be used for Construction unless signed as Approved	A1 Drawing No: 51-31468-C215

Plot Date: 9 June 2014 - 4:39 p.m. Plotted by: Nicki Mackie Cad File No: G:\51\31468 Whakatu\CADD\Drawings\51-31468-C215_C217.dwg



Rev: B

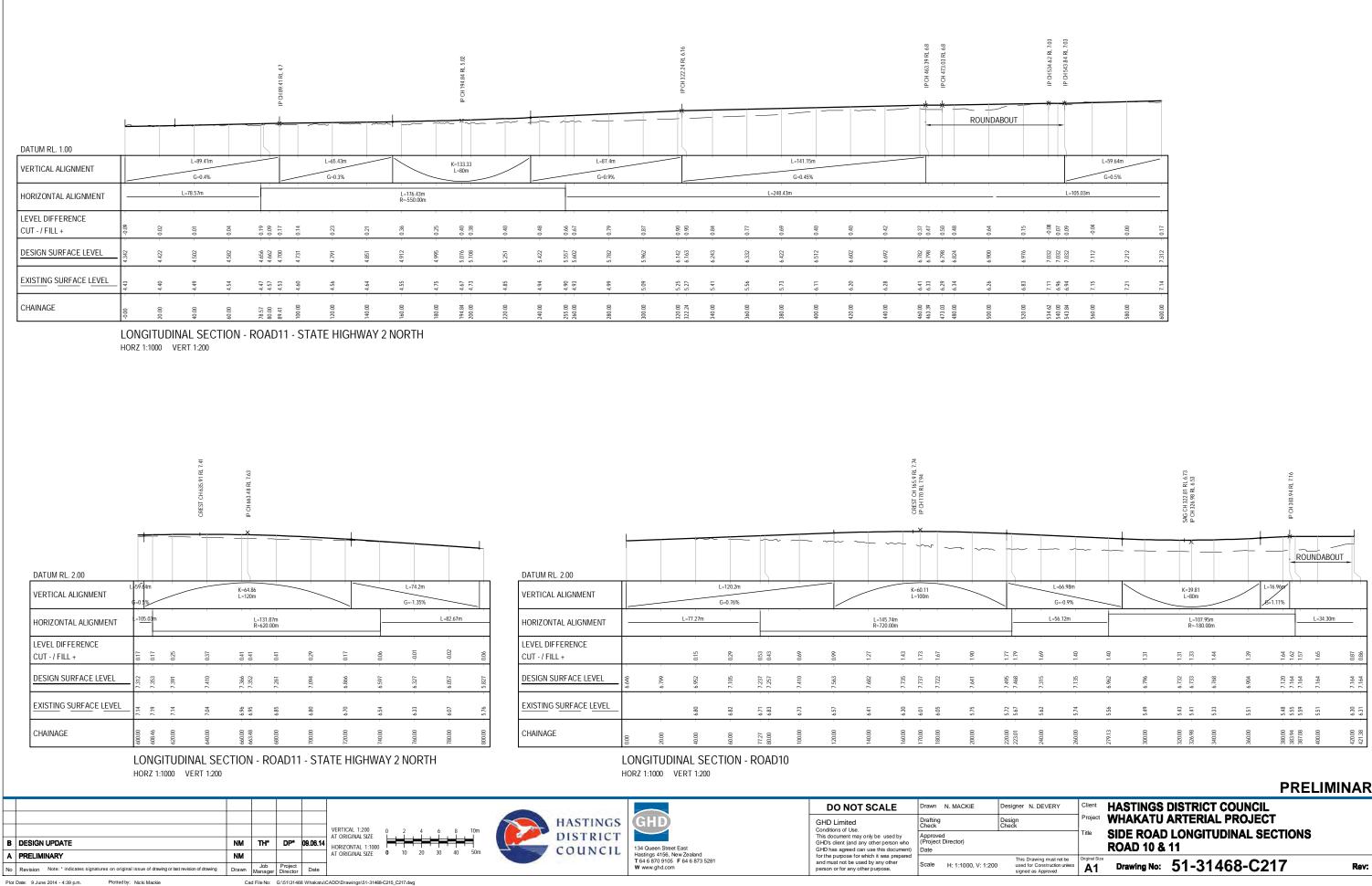


7.18

No Revision Note: " indicates signatures on original issue of drawing or last revision of drawing Drawn Manager Director Date Plot Date: 9 June 2014 - 4:39 p.m. Plotted by: Nicki Mackie Cad File No: G:\51\31468 Whakatu\CADD\Drawings\51-31468-C215_C217.dwg

PRELIMINARY

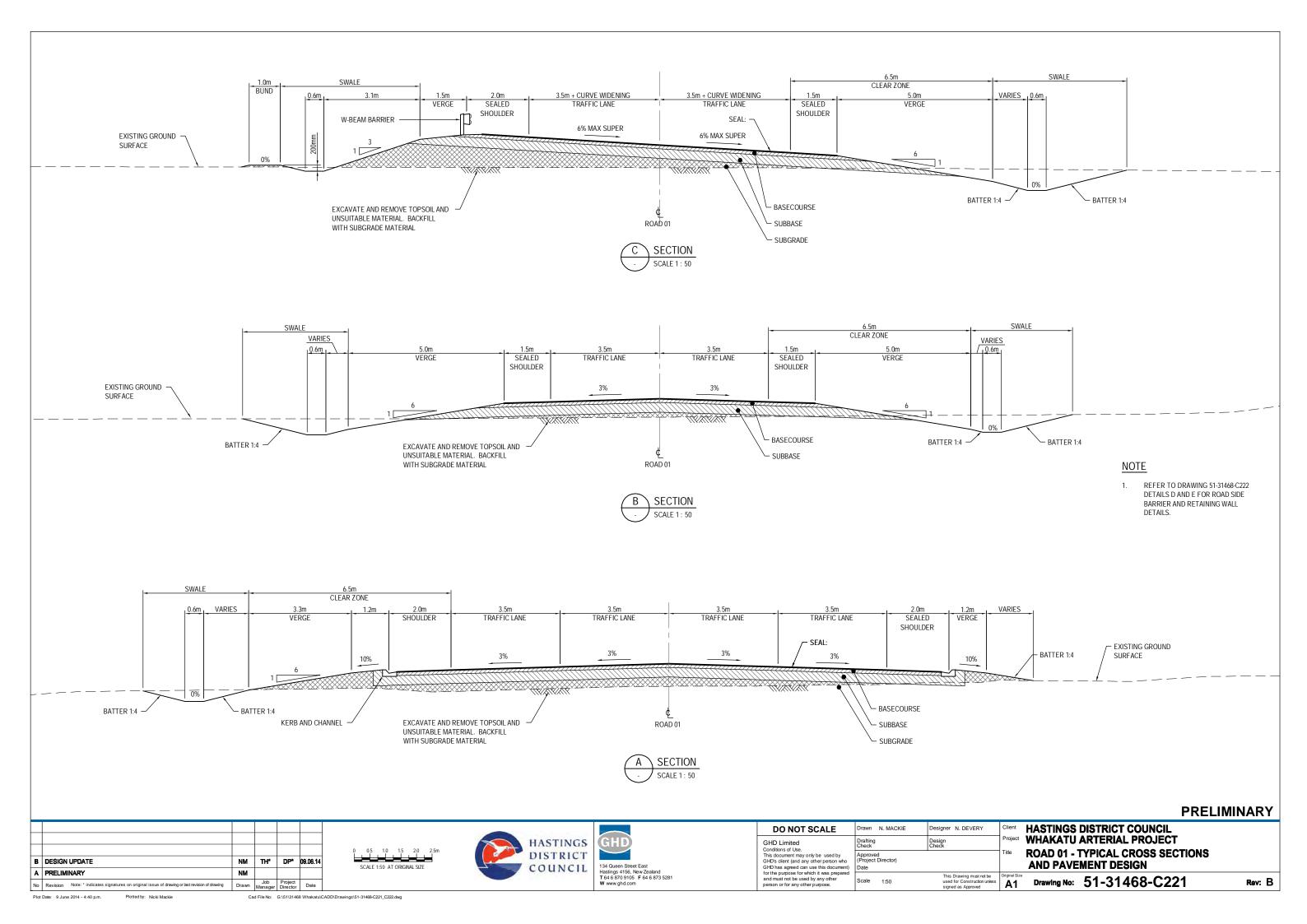
(DISTRICT COUNCIL	
			ARTERIAL PROJECT	
	Title	SIDE ROAD ROAD 07	LONGITUDINAL SECTIONS	
not be n unless	Original Size	Drawing No:	51-31468-C216	Rev: B

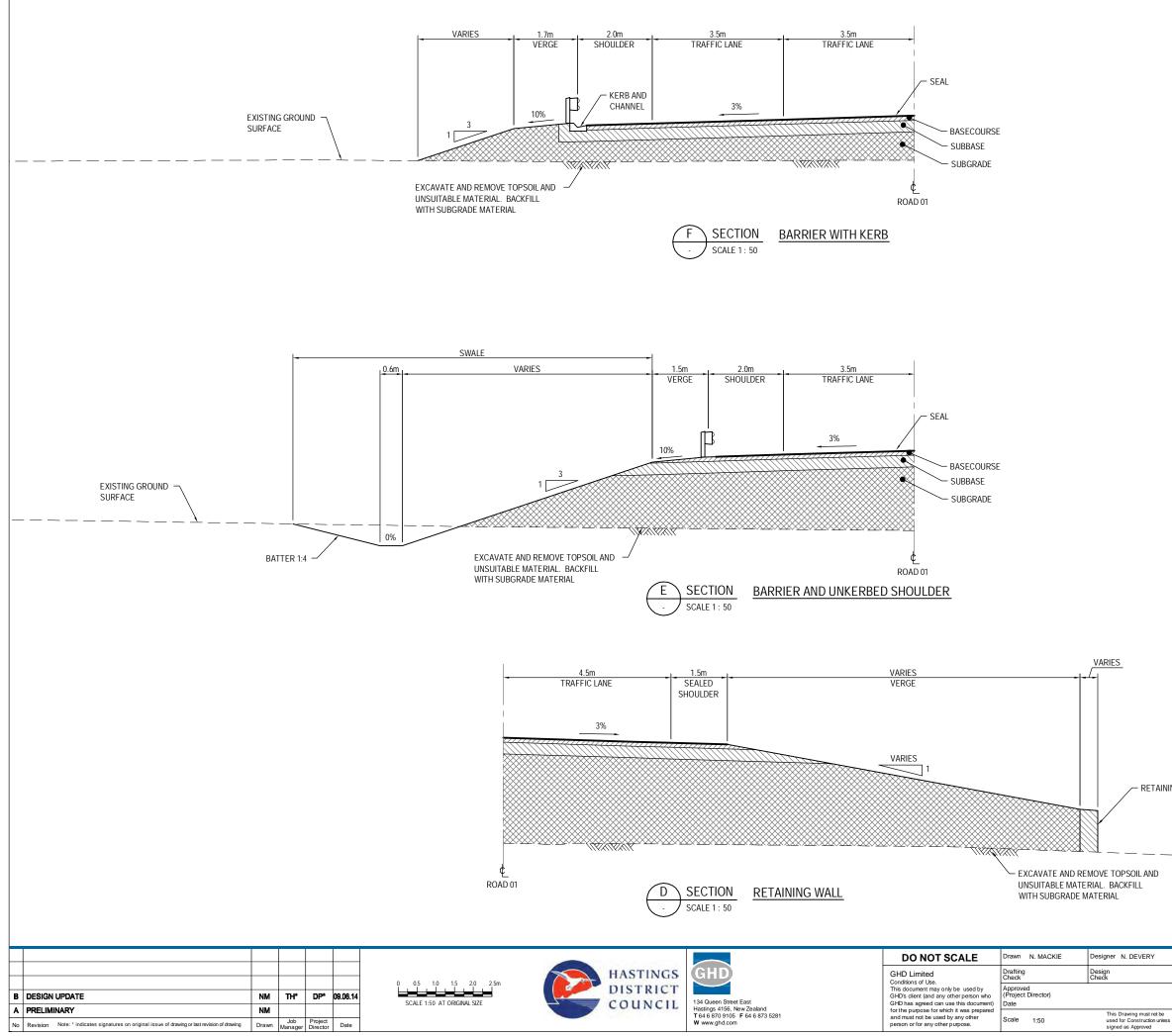




K			
\			
1	Ŀ	=59.64m	
		G=0.5%	
-			1
L=105.0)3m		
			-
6	-0.04	0.00	1
- 0.09			- 0.17
7.032	7.112	7 212	7.312
-	-	-	-
.94	7.15	7.21	7.14
6.94	-		
	560.00	Q	00.008
			9
543.84	.099	580.00	00

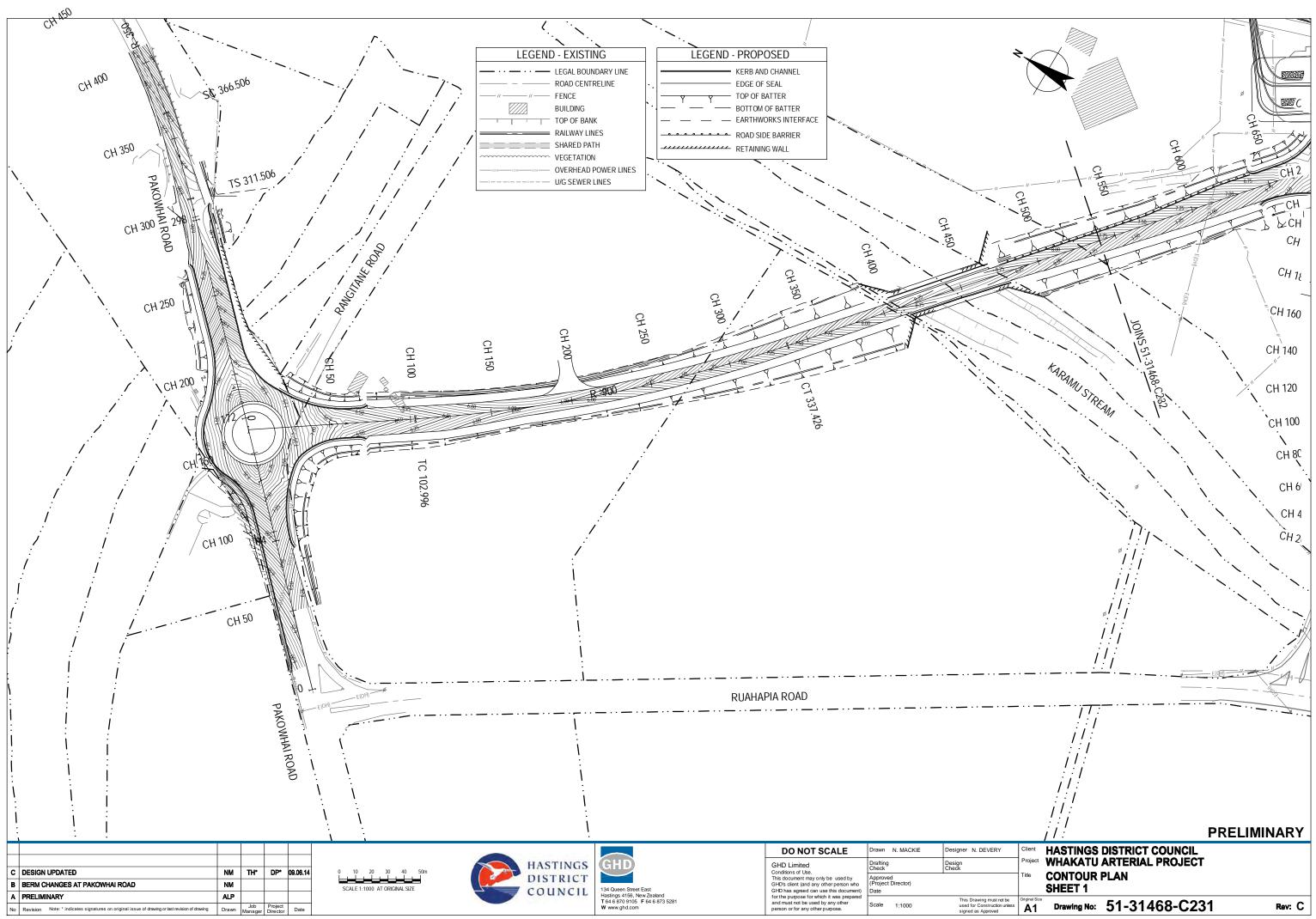
be unless	Original Size	Drawing No:	51-31468-C217	Rev: B
	Title	SIDE ROAD ROAD 10 &	LONGITUDINAL SECTIONS 11	
	Client Project	WHAKATU	DISTRICT COUNCIL ARTERIAL PROJECT	
			PREL	MINARY

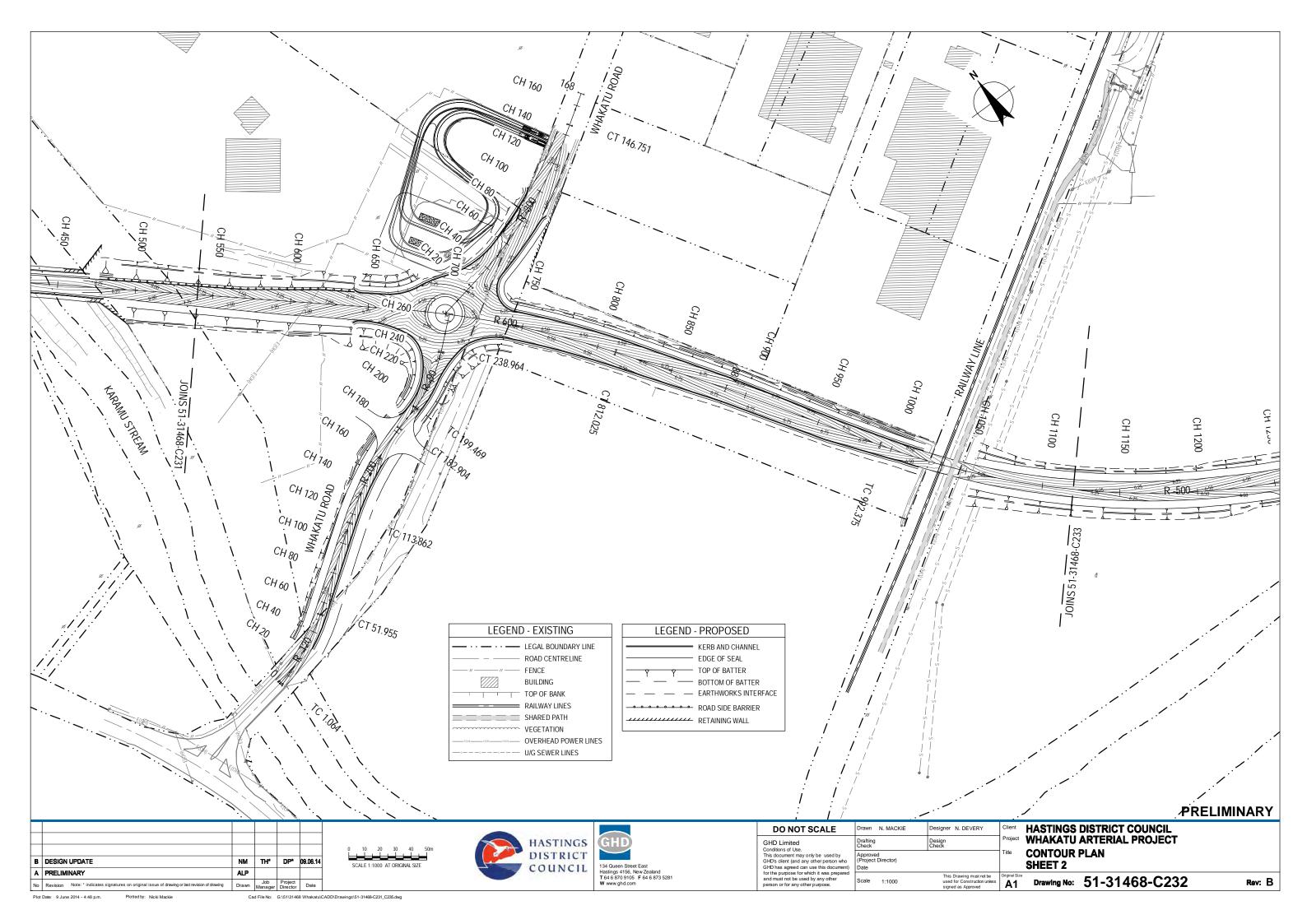


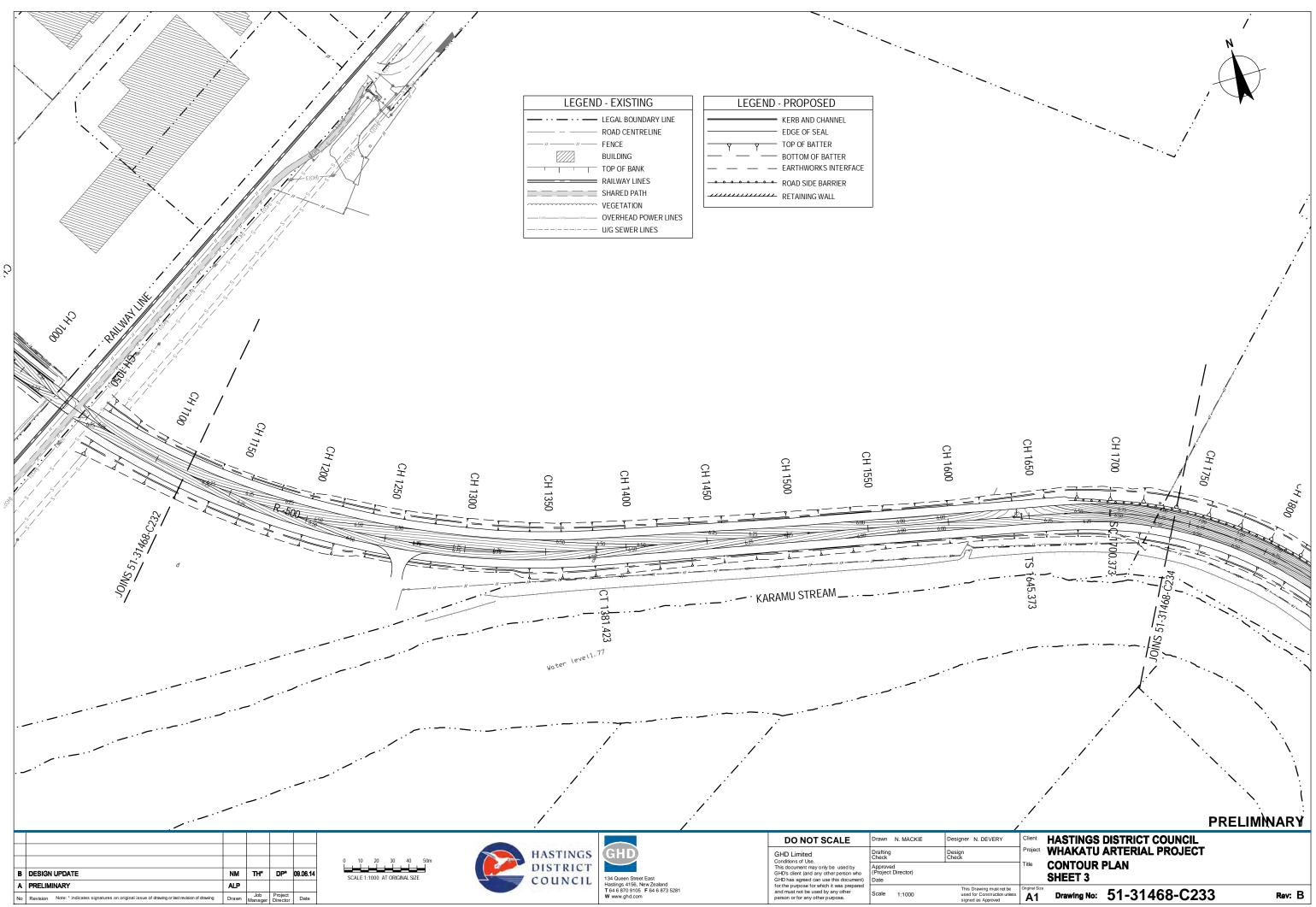


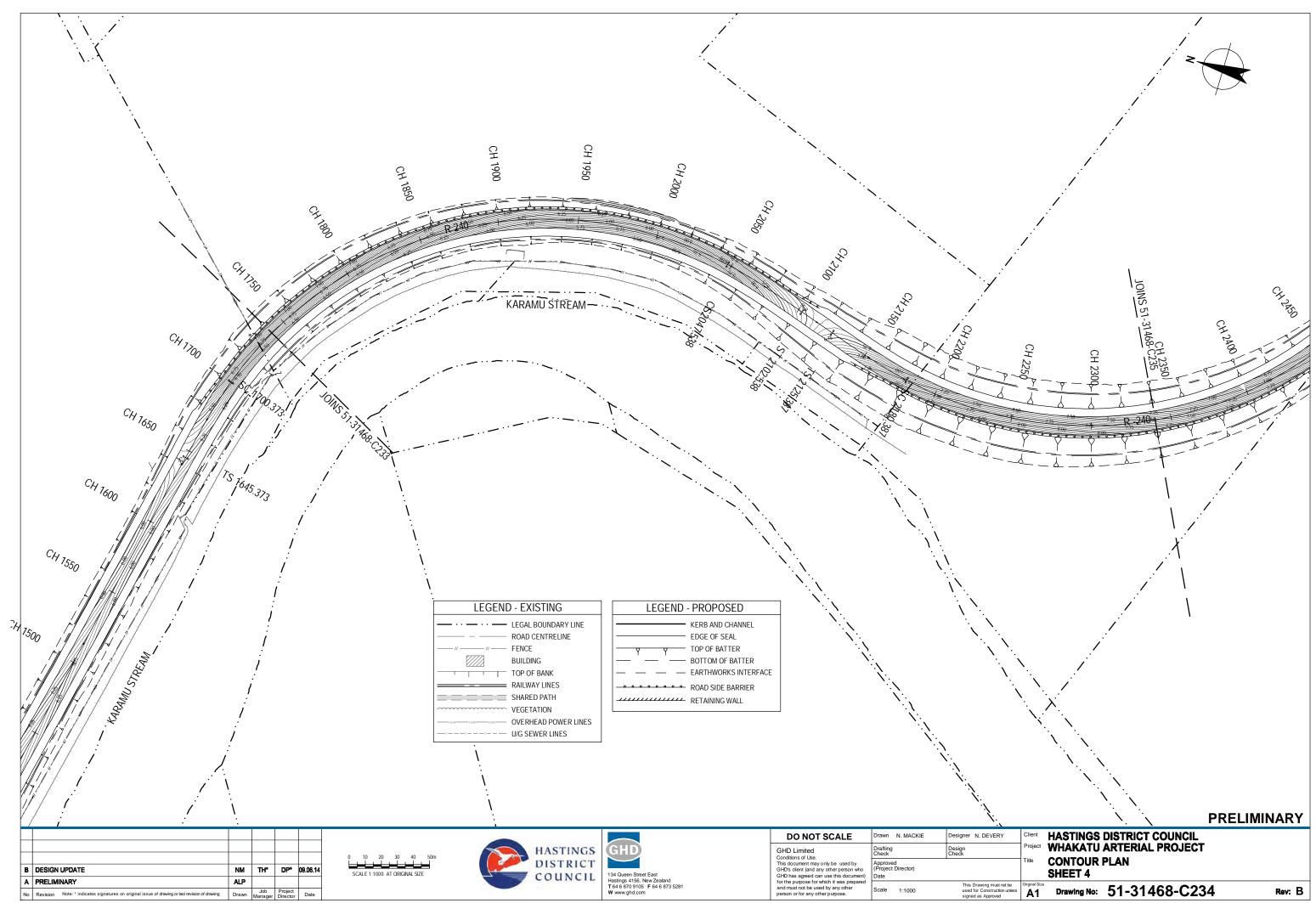
Plot Date: 9 June 2014 - 4:40 p.m. Plotted by: Nicki Mackie Cad File No: G:51\31468 Whakatu\CADD\Drawings\51-31468-C221_C222.dwg

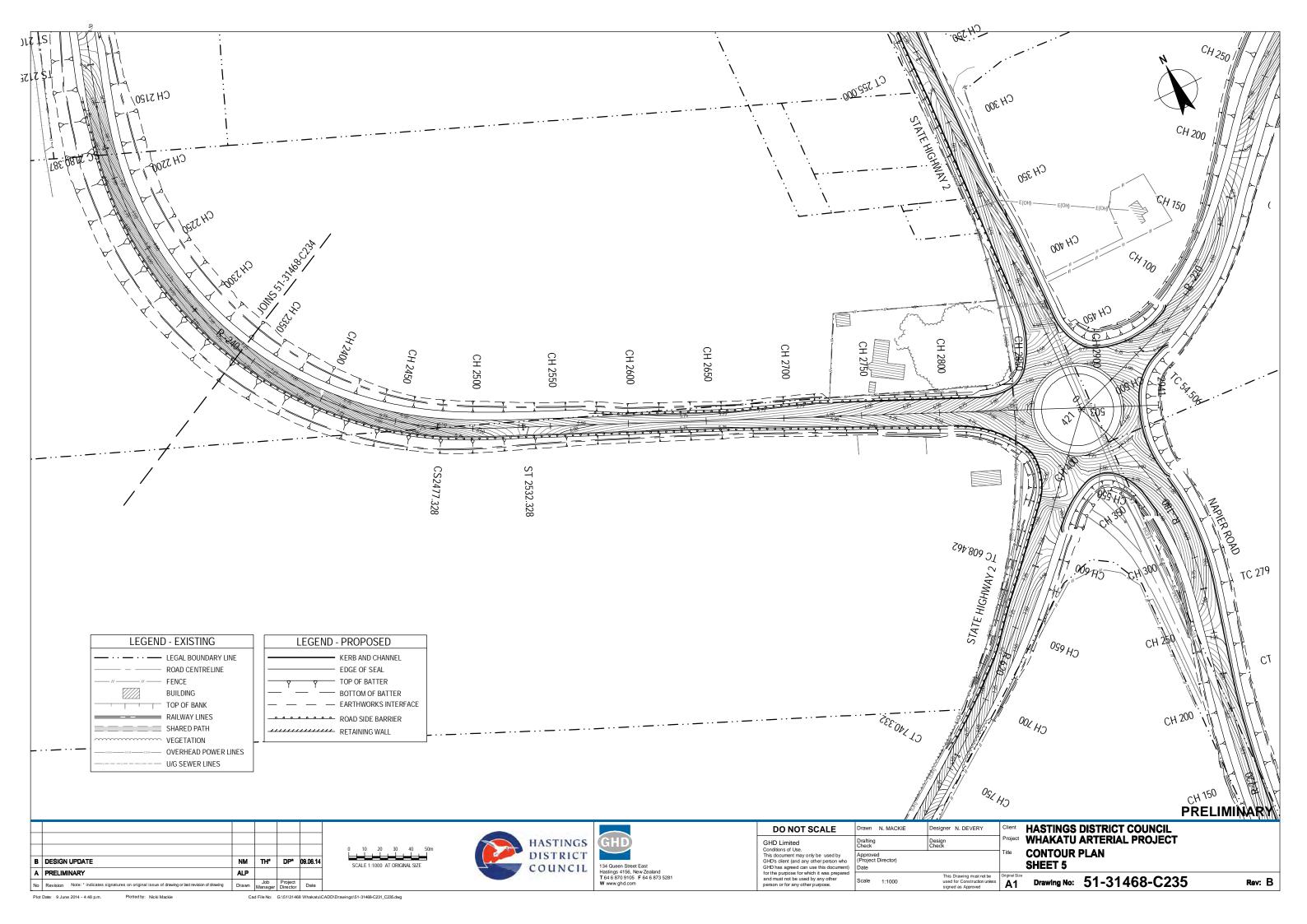
- RETAINING WALL - EXISTING GROUND SURFACE PRELIMINARY HASTINGS DISTRICT COUNCIL WHAKATU ARTERIAL PROJECT **ROAD 01 - TYPICAL CROSS SECTIONS** Title AND PAVEMENT DESIGN Drawing No: 51-31468-C222 Rev: B Â1

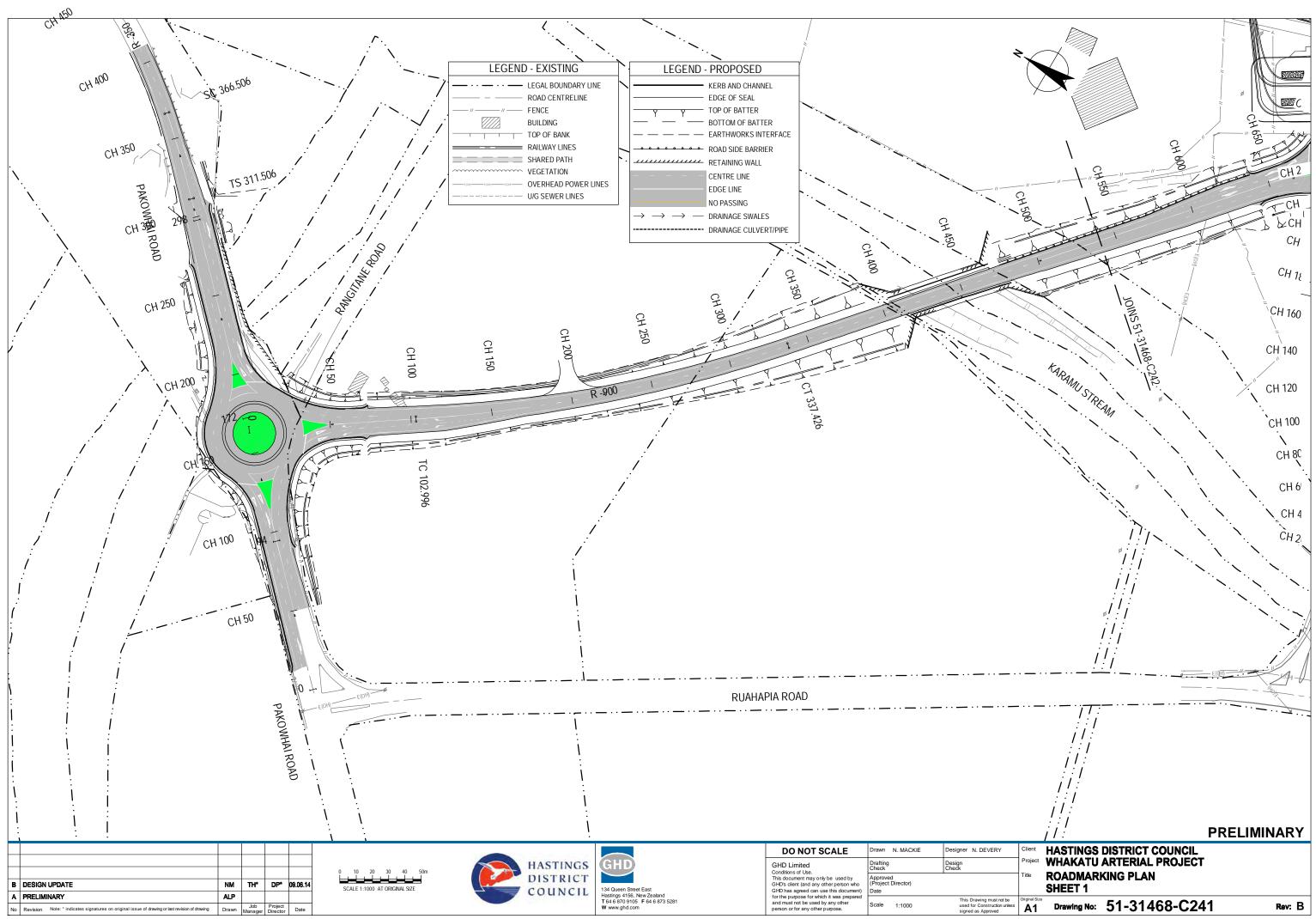




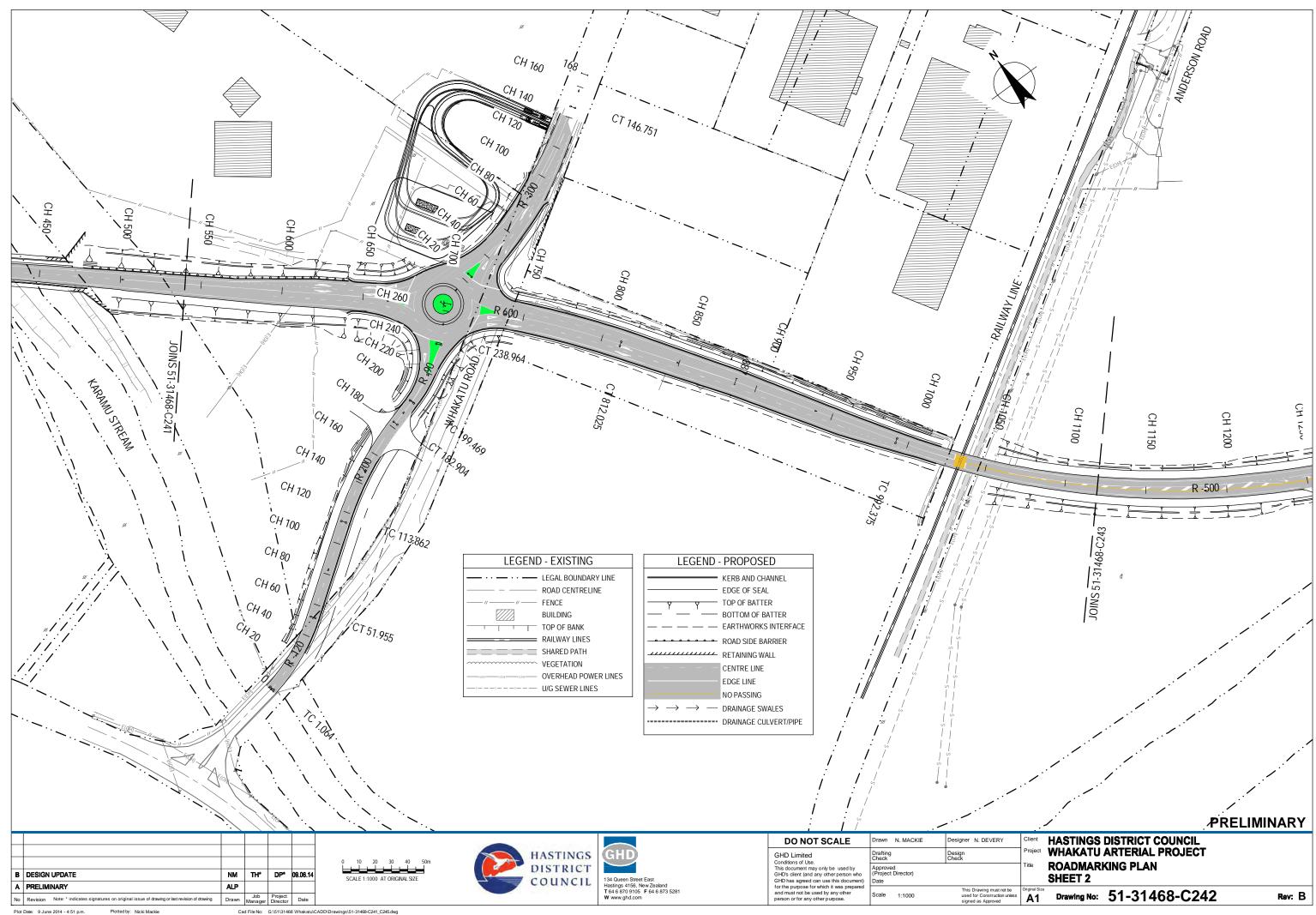


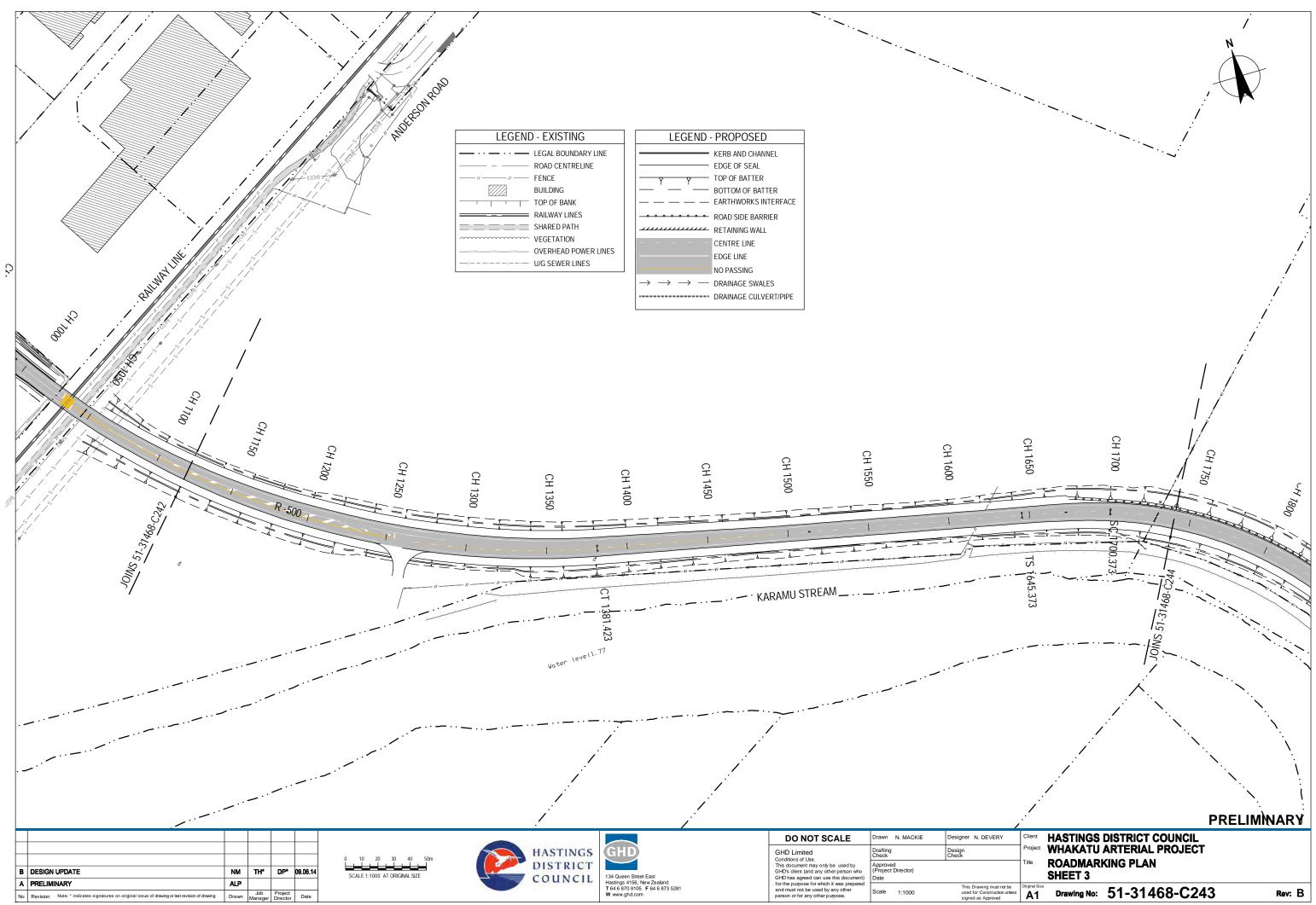


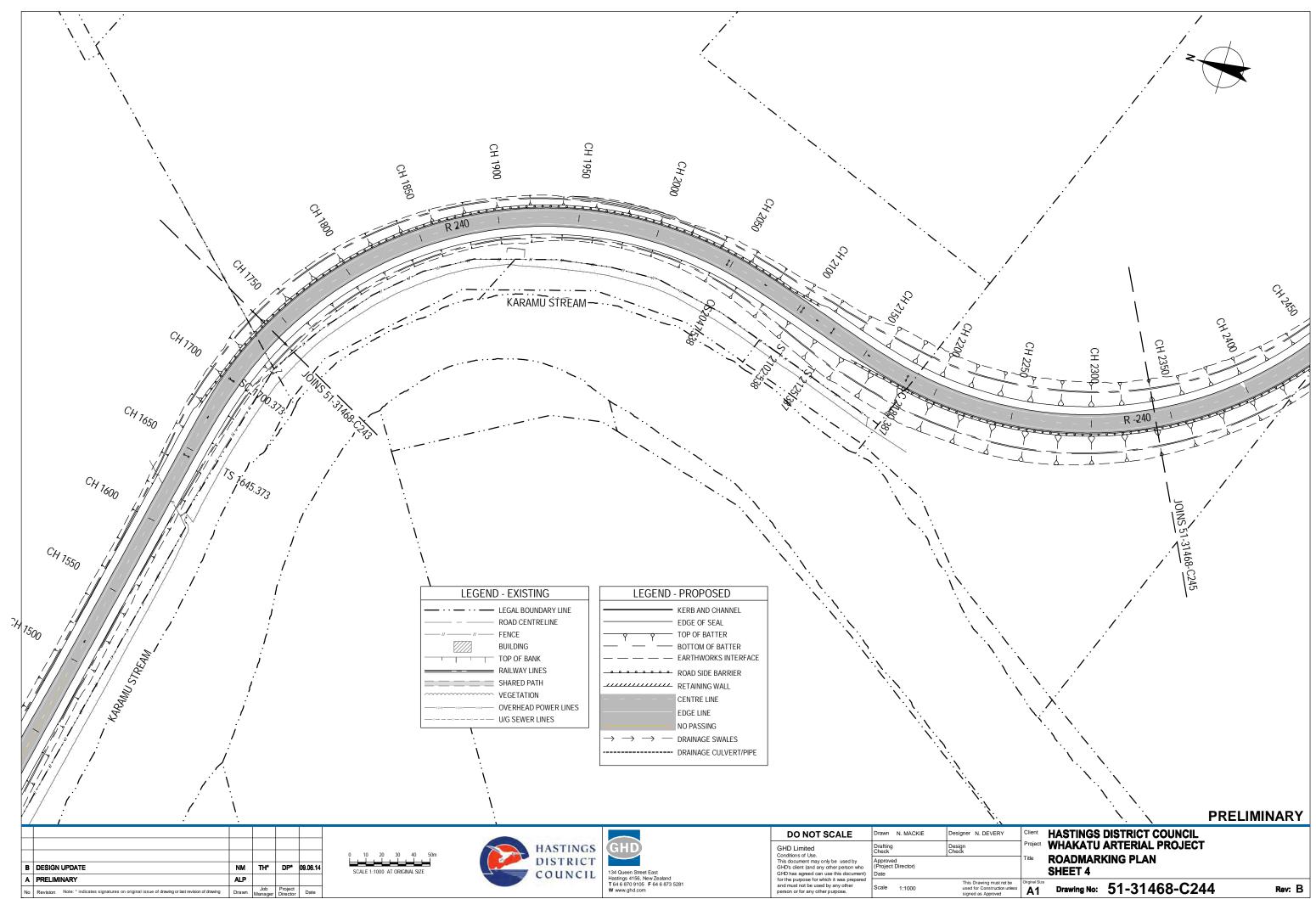


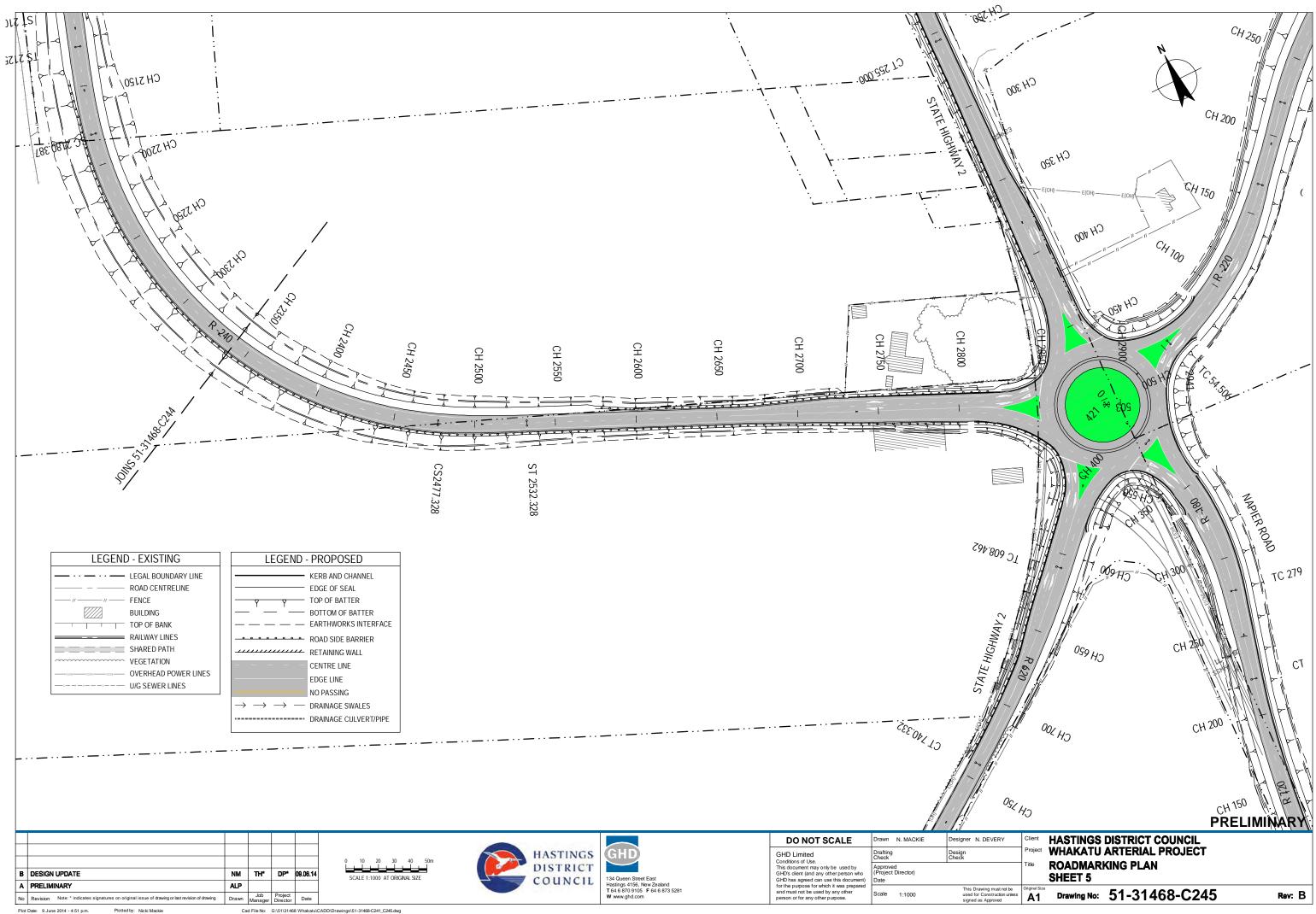


Plot Date: 9 June 2014 - 4:51 p.m. Plotted by: Nicki Mackie Cad File No: G\51\31468 Whakatu\CADD\Drawings\51-31468-C241_C245.dwg









Plot Date: 9 June 2014 - 4:51 p.m. Plotted by: Nicki Mackie

		1 in -6 -3% -3%	-3% 1 in -6	1 in
DATUM R.L. 3.00				
DESIGN SURFACE LEVEL	4.935 5.119 5.119 5.119 5.022 5.022	5.901 5.901 5.909 6.014	5.909	5.03 4.694 4.928 4.928
DEPTH CUT - / FILL +	-0 0.19 0.19 -0.01 -0.01 -0.01 0.09	0.92 0.96 0.97	0.95	009 -024 -024
EXISTING SURFACE LEVEL	4,93 4,93 4,93 4,93 4,93 4,93	4.94 4.94 4.94 4.95	4.95	4.94 4.93 4.93 4.93
OFFSET FROM CENTRELINE		-5.27 3.77 3.5	2 22	10 11.35 11.95 12.88

DATUM R.L. 3.00	0% <u>10.6</u> - <u>3% <u>10.6</u> <u>10.4</u></u>
DESIGN SURFACE LEVEL	5,122 5,172 5,517 5,517 5,517 5,607 5,518 5,528
DEPTH CUT - / FILL +	01 01 01 01 008 088 088 088 088 092 008 089 092 008 008 008 008 008 008 008 008 008 00
EXISTING SURFACE LEVEL	5,5,2,5,5,5,5,5,5,5,5,5,5,5,5,5,5,5,5,5
OFFSET FROM CENTRELINE	14 43 14 43 14 43 11 44 11 12 13 13 13 49 49 49 49 49 49 49 49 49 49 11 4 11 4

CH 150

CH 100

6.818 6.835

1.29

5.53

-0.45 0.58

CH 50

1.2%

-9.5% 1 in -6.3

.163

.56

6.729 6.729 6.795 6.699

1.15 1.15 1.21 1.11

5.58 5.58 5.59 5.59

9.28 9.28 9.63 10.75

	1 in <u>4 1 in 6</u>	-3%		-3%	<u>1 in -6</u> 1-in a	-1 in 4
DATUM R.L. 3.00						
DESIGN SURFACE LEVEL	5.174 5.316 5.316 5.116 5.116 5.373 5.373	6 207 6 252 6.34	6.445	6.34 6.235 6.19	5.357	5.524
DEPTH CUT - / FILL +	-0 0.14 0.14 0.13 0.13 0.13	0.8 0.8 0.79	0.83	0.53	-0.29	Х Ф С
EXISTING SURFACE LEVEL	5.17 5.17 5.17 5.19 5.21 5.24	5.45 5.45 5.55	5.62	5.71 5.71	5.65	5.52
OFFSET FROM CENTRELINE	-16.98 -16.39 -15.39 -15.39 -13.45 -12.96	-7.96 -6.46 -3.5	0	3.5 7 8.5	13.5 15.34	

1 in -6.3 -9.5%

6.555 6.651 6.585 6.585

1.07 1.17 1.11 1.11

5.49 5.48 5.48 5.48

-11.61 -10.49 -10.13

5.538 5.278 5.278

-0 -0.25 -0.25

5.54 5.53 5.53

19.77 18.68 18.06

.017

0.51

5.51

14.98

DATUM R.L. 3.00 DESIGN SURFACE LEVEL 4.722 5.287 6.774 6.819 DEPTH CUT - / FILL + -0.63 1.63 EXISTING SURFACE LEVEL 5.35 5.29 5.14 OFFSET FROM CENTRELINE -17.74 -15.48 -14.88 -3.5

4.956 6.611 6.649

0 1.66 1.68

	 	1			in 4	1 in -6
 DATUM R.L. 3.00						
DESIGN SURFACE LEVEL	4.907	5.02	20.C	4.82	5.323	
DEPTH CUT - / FILL +	0	0.11	0.00	-0.09	0.41	
EXISTING SURFACE LEVEL	4.91	4.91	4.9	4.91	4.91	
OFFSET FROM CENTRELINE	-14.86	-14.41	- 13.41	-12:01	-10	

						0 2 4 6
в	DESIGN UPDATE	NM	TH*	DP*	09.06.14	SCALE 1:200 AT ORIG
A	PRELIMINARY	NM				
No	Revision Note: * indicates signatures on original issue of drawing or last revision of drawing	Drawn	Job Manager	Project Director	Date	
Plot	Date: 9 June 2014 - 4:54 p.m. Plotted by: Nicki Mackie	Ca	d File No:	G:\51\3146	B Whakatu\	CADD\Drawings\51-31468-C251_C258.dwg

DATUM R.L. 3.00 DESIGN SURFACE LEVEL

DEPTH CUT - / FILL +

EXISTING SURFACE LEVEL

OFFSET FROM CENTRELINE

4 6 SCALE 1:200 AT ORIGINAL SIZE



4.997

-0.6 -0.6

5.6 5.6 5.57

18.97 19.6 22.03

575

S	GHD
L	134 Queen Street East Hastings 4156, New Zealand T 64 6 870 9105 F 64 6 873 5281 W www.ghd.com

						PRELIMINARY
DO NOT SCALE	Drawn N. MACKIE	Designer N. DEVERY	Client		DISTRICT COUNCIL	
GHD Limited Conditions of Use.	Drafting Check	Design Check	Project		ARTERIAL PROJECT	
This document may only be used by GHD's client (and any other person who GHD has agreed can use this document)	Approved (Project Director) Date	Title	CROSS SECTIONS - ROAD01 CH 50 TO CH 400			
for the purpose for which it was prepared and must not be used by any other person or for any other purpose.	Scale 1:200	This Drawing must not be used for Construction unless signed as Approved	Original Size	Drawing No:	51-31468-C25	51 Rev: B

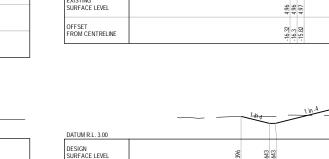
	1 in 6										
		<u>h4</u> 1in.4						<u>1 in 4</u> 1 in	<u> </u>		
DATUM R.L. 3.00											
DESIGN SURFACE LEVEL	5.396	4.643	6.616	7.449	7.599	7.494 7.449	6.616	4.496 4.496	5.376		
DEPTH CUT - / FILL +	0	-0.74 -0.73	12	2.11	2.29	2.13 2.08	1.25	-0.87	ę		
EXISTING SURFACE LEVEL	24	5.39	5.42	5.34	5.31	5.37	5.37	5.37	238		
OFFSET FROM CENTRELINE	-21.51	-18.49 -17.89	-10	-5 -3.5	0	3.5	10	18.48	22.6		

8.025

2.85

5.22 5.22

-3.5



DATUM R.L. 2.00 DESIGN SURFACE LEVEL

DEPTH CUT - / FILL +

EXISTING SURFACE LEVEL

	-370	_	<u>1 in -6</u>				
				<u>1 h.4</u>		<u>1in4</u>	
8.175	8.07	8.025	7.192	4.446	4.446	5.244	
2.96	2.86	2.82	1.99	82.0-	-0.78	Ģ	
5.21	5.21	5.21	5.2	5.22	5.23	5.24	
0	3.5	5	10	20.98	21.58	24.77	

CH 400

-3%

CH 350

	-3%		1 in -6				
				<u>1 in -4</u>		1 in 4	
6.924	6.819	6.774	5.941	4.546	4.546	5.109	
1.88	1.75	1.7	0.85	-0.56	-0.56	0	
5.05	5.07	5.07	5.09	5.1	5.1	5.11	
0	3.5	5	10	15.58	16.18	18.43	

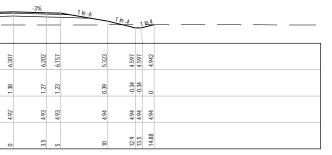
CH 300

6.157 6.202

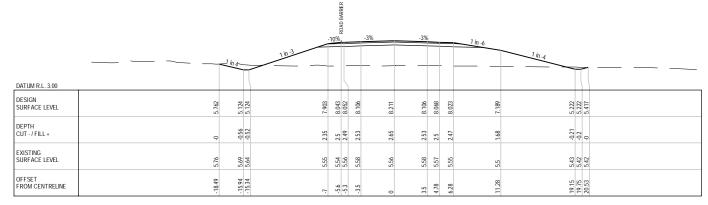
1.24

4.92

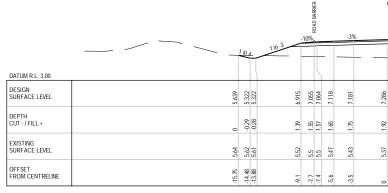
-3.5

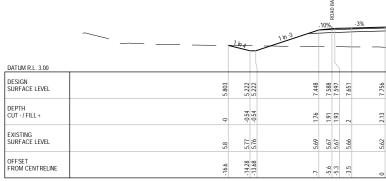


CH 250



DATUM R.L. 3.00		1 in 4 1	<u>n-4 1 in-6 1</u>	n-10 -4%	-3%		-3%	<u>1 in :10 - 7 in -0</u>	1 in -4	0.6%
DESIGN SURFACE LEVEL	500 7	5.598 5.598	5.955 6.505	6.605 6.552 6.54 6.54 6.54	6.711	6.711	6.606	6.552 6.556 6.556 6.621 6.521	5.97 4.698	4.097 6.827 6.834
DEPTH CUT - / FILL +		-0.83	-0.5	0.1 0.04 0.03 0.03	0.13	61.0	-0.09	-0.16 -0.16 -0.16 -0.1 -0.1	-0.8 -2.14	0.00
EXISTING SURFACE LEVEL	6	6.43 6.44	6.45 6.5	6.51 6.51 6.51 6.53 6.53	6.58	6.66	699	6.71 6.71 6.72 6.73 6.73	6.77 6.84 6.84	684
OFFSET FROM CENTRELINE	5	-15.42 -14.82	-13.39 -10.09	-8.99 -8.8 -8.64 -8.64 -7	-3.5	3.5	7	8.8 9.09 9.44 10.54	13.84 18.94	27.02





									DO NOT SCALE	Drawn N. MACKIE	Designer N. DEVER'
-				_		0 2 4 6 8 10m	HASTINGS	GHD	GHD Limited Conditions of Use.	Drafting Check	Design Check
E	3 DESIGN UPDATE	NM	TH*	DP*	09.06.14	SCALE 1:200 AT ORIGINAL SIZE	DISTRICT	134 Queen Street East	This document may only be used by	Approved (Project Director)	
4	A PRELIMINARY	NM					COUNCIL	Hastings 4156, New Zealand T 64 6 870 9105 F 64 6 873 5281	for the purpose for which it was prepared and must not be used by any other	Scale 1:200	This Drawing must
N	o Revision Note: * indicates signatures on original issue of drawing or last revision of drawing	Drawn	Job Manage	Project Directo	r Date			W www.ghd.com	person or for any other purpose.	1:200	used for Construction signed as Approved

				PRELIMINARY	
(Client		DISTRICT COUNCIL		
	Project	WHAKATU	ARTERIAL PROJECT		l
	Title	CROSS SE	CTIONS - ROAD01		l
		CH 450 TO	CH 700		
not be n unless	Original Size	Drawing No:	51-31468-C25	2 Rev: B	

CH 550

1		3%		1 in -6				
					<u>1 in 4</u>	_	1 in 4	
	7.651	7.577	7.523	6,69	4.998	4.998	5.44	
	2.06	2.01	1.98	1.19	-0.45	-0.45	0	
40.0	5.59	5.56	5.55	5.5	5,45	5.44	5.44	
,	3.5	5.95	7.75	12.75	19,52	20.12	21.89	

CH 600

					<u>1 in 4</u>		1		
-	7.181	7.076	7.022	6.189	4.848	4.848	5.048	5.048	4.88
-	1.89	1.85	1.83	1.16	-0.01	-0.01	0.18	0.17	0
	5.3	5.23	5.19	5.03	4.86	4.86	4.86	4.87	4.88
,	3.5	7	8.8	13.8	19.16	19.76	20.56	21.56	22.23

1 in -6

CH 650

	<u>1in-4</u>	<u>1 in -6 -10%</u>	-3%		-3%	-	-10	<u>% 1 in</u>	<u></u>
ATUM R.L. 4.00									
IESIGN URFACE LEVEL	5.708 5.664 5.664 6.147	6.697 6.797 6.732 6.732	6.785 6.785 6.83	6.935	6.83	6.725	6.673 6.673 6.73	6.638	6.222
NEPTH JUT - / FILL +	0 -0.1 0.23	0.75 0.84 0.77	0.81	0.9	0.74	0.58	0.5 0.5 0.5	0.45	0-
XISTING URFACE LEVEL	5.71 5.76 5.9 5.92	5.95 5.96 5.96	5.97 5.99 5.99	6.03	6:09	6.14	6.17 6.17 6.17	6.19	6.22
IFF SET ROM CENTRELINE	-14.23 -14.05 -13.45 -11.52	-8.22 -7.12 -6.77	-6.// -5.02 -3.5	0	3.5	7	8.75 8.75 0.1	10.2	12.69
			<u> </u>	CH 90	m	7	00 00 0	<u>-1</u>	2

	7,		1 in -6 -3		-3% 1 in	-6		
	 	14 1114				0 T in -4	1 11 4	
DATUM R.L. 3.00								
DESIGN SURFACE LEVEL	5.534	4.867 4.867 5.229	6.063	6.213 6.23	6.125 6.08	5.246 4.469 4.469	5.662	
DEPTH CUT - / FILL +	0	-0.69 -0.69 -0.34	0.47	0.6	0.5 0.45	-0.4 -1.19 -1.19	0	
EXISTING SURFACE LEVEL	5.53	5.55 5.56 5.57	5.6	5.61	5.62	5.65 5.66 5.66	5.66	
OFFSET FROM CENTRELINE	-15.28	-12.61 -12.01 -10.56	-5.56 -4.06	-0.56 0	3.5	10 13.11 13.71	18.48	

		. 1 in -6		-3%	-3%	1 in -6	
DATUM R.L. 4.00		<u>m.4 1in.6</u>					
DESIGN SURFACE LEVEL	5.709 5.709 5.763 5.563 5.563 5.563	5.999 6.540		6.717 6.822	6.717 6.612	6.56 6.56 6.555 6.525 6.525 5.975 5.975 5.962	
DEPTH CUT - / FILL +	0 003 003 -018 -018	0.22	0.8 0.73 0.73 0.79	0.92	0.85	0.64 0.64 0.7 0.59 0.01	
EXISTING SURFACE LEVEL	5.71 5.73 5.73 5.75	5.78	5.85 5.85 5.85 5.85	5.83 5.83	5.87	5.92 5.92 5.93 5.94 5.96 5.96	
OFF SET FROM CENTRELINE	-17.04 -15.83 -15.83 -15.03	-12.68 -0.38	-8.28 -7.93 -7.93 -6.18	-3.5	3.5	8.75 8.75 9.1 10.2 13.5	

	-	-3%	-1.2%	-0.4%	-3	3%
DATUM R.L. 4.00	=					=
DESIGN SURFACE LEVEL	5	72	315		10	48
	6.22	6.272	6.3		6.301	6.248
DEPTH CUT - / FILL +	0.54	0.59	0.6		0.55	0.48
EXISTING SURFACE LEVEL	5.68	5.69	5.71		5.75	5.77
OFFSET FROM CENTRELINE	-5.25	-3.5	0		3.5	5.25

	1 in 6
DATUM R.L. 4.00	
DESIGN SURFACE LEVEL	5 5 926 6 6 6083 5 926 6 443 5 983 5 983 5 983 6 443 6 443 6 443 6 6 6 6 8 8 6 6 8 8 6 6 8 8 7 8 8 6 7 8 8 6 6 8 8 7 8 8 6 6 8 8 7 8 8 8 8
DEPTH CUT - / FILL +	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
EXISTING SURFACE LEVEL	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
OFFSET FROM CENTRELINE	$\begin{array}{cccccccccccccccccccccccccccccccccccc$

CH 1010

DATUM R.L. 4.00	<u>0% 1h.4 1h.6 10% 3% 3% 3% 1h.6</u>	
DESIGN SURFACE LEVEL	6 5 933 5 934 5 944 5 944 6 138 6 13	
DEPTH CUT - / FILL +	0.011 0.011 0.012 0.014 0.014 0.014 0.014 0.014 0.014 0.014 0.014 0.014 0.014 0.014 0.014 0.011 0.	
EXISTING SURFACE LEVEL	553 558 558 558 558 558 558 653 602 613 613 613 613 613 613 613 613 613 613	
OFFSET FROM CENTRELINE		

CH 950

		1	1	i	1	
в	DESIGN UPDATE	NM	тн*	DP*	09.06.14	
A	PRELIMINARY	NM				
No	Revision Note: * indicates signatures on original issue of drawing or last revision of drawing	Drawn	Job Manager	Project Director	Date	

2 4 6 8 10m CALE 1:200 AT ORIGINAL SIZE

Cad File No: G:\51\31468 Whakatu\CADD\Drawings\51-31468-C251_C258.dwg



DO NOT SCALE	Drawn N. MACKIE	Designer N. DEVERY
GHD Limited	Drafting Check	Design Check
This document may only be used by GHD's client (and any other person who GHD has agreed can use this document)	Approved (Project Director) Date	
for the purpose for which it was prepared and must not be used by any other person or for any other purpose.	Scale 1:200	This Drawing must not b used for Construction un signed as Approved

Plot Date: 9 June 2014 - 4:54 p.m. Plotted by: Nicki Mackie

-0.49	%	-3%
_	_	_

CH 1045

PRELIMINARY

	Project Title	ARTERIAL PROJECT CTIONS - ROAD01	
be inless	Original Size	 51-31468-C253	Rev: B

DATUM R.L. 3.00	 <u>1-in</u>	4	<u>1 in 4</u>	1 in -6		-3%		-3%		<u>1 in -6</u>	1 ii	<u>n.4.1</u>	in 4	
DESIGN SURFACE LEVEL	5.733	5.059	5.702	6.535	6.58	6.685	6.783	01.4.4	01000		5.8	5.443 5.443	5.799	
DEPTH CUT - / FILL +	9	-0.68 -0.68	-0.04	0.78	0.83	0.92	1.03	au	0.00	6	10:0	-0.35	0	
EXISTING SURFACE LEVEL	5.73	5.74 5.74	5.74	5.75	5.75	5.76	5.76	£ 70	02.0		6/.c	5.79 5.8	5.8	
OFFSET FROM CENTRELINE	-19.15	-16.45 -15.85	-13.28	-8.28	-6.78	-3.28	0	ц	с, 1		<u>0</u>	11.43 12.03	13.45	

		<u>in 4 1 in 4</u>	1 in -6	=	-3%		-3%		<u>1 in -6 1</u>		in 4	 	
DATUM R.L. 3.00													
DESIGN SURFACE LEVEL	5.621	5.01 5.01 5.47e		6.504	6.609	6.714	6.609	6.564	5.73	5.341 5.341	5.802		
DEPTH CUT - / FILL +	0	-0.62	0.79	0.83	6:0	0.98	0.85	0.8	-0.06	-0.46 -0.46	0		
EXISTING SURFACE LEVEL	5.62	5.63 5.63 5.63	5.67	5.68	5.7	5.73	5.75	5.76	5.79	5.8 5.8	5.8		
OFFSET FROM CENTRELINE	-19	-16.56 -15.96	-8.5	Ŀ	-3.5	0	3.5	2	10	11.56 12.16	22		

	 	14	1 in -6	-3	%	-3%	_	<u>lin-6</u>	in 4	 	
DESIGN SURFACE LEVEL	5.903	5.206 5.206 5.403	6.236	6.281	6.386 6.407	6.302	6.257	5.424 5.199 5.199	6.065		
DEPTH CUT - / FILL +	0	-0.71 -0.71 -0.51	0.32	0.37	0.47	0.37	0.32	-0.58 -0.83 -0.85	0		
EXISTING SURFACE LEVEL	5.9	5.91 5.91 5.91	5.91	5.91	5.92 5.92	5.93	5.93	6.01 6.03 6.05	6.07		
OFFSET FROM CENTRELINE	-14.89	-12.1 -11.5 -10.72	-5.72	4.22	-0.72 0	3.5	2	10 11.5	14.96		

1 in -6

6.108 6.153

0.14

5.97

3.5

1 in 4

5.917

5.92

14.21

5.145 5.145 5.274

-0.79 -0.8 -0.67

5.94 5.94 5.94

-11.12 -10.52 -10

CH 1450

		_1 in 4	m-4	1 in -6		-3%		-3%		<u>1 in -6</u>	<u>th-4</u>	Tin4	
DESIGN SURFACE LEVEL	5.85	5.156 5.156	5.527	6.36	6.405	6.51	6.557	6.452	6.407	5.574	5.099	5.099	
DEPTH CUT - / FILL +	0	-0.69 90.0-	-0.32	0.49	0.53	0.61	0.65	0.52	0.46	-0.39	-0.88	-0.88	
EXISTING SURFACE LEVEL	بې عز	5.85	5.85	5.87	5.88	5.9	5.91	5.93	5.94	5.97	5.98	5.98	
OFFSET FROM CENTRELINE	-16.43	-13.65 -13.05	-11.57	-6.57	-5.07	-1.57	0	3.5	5	10	11.9	12.5 16.07	

CH 1400

				1 in -6		-3%		
	 	14-1	in A		-			-
DATUM R.L. 3.00								
DESIGN SURFACE LEVEL	5.766	5.107 5.107	5.651	6.485	6.53	6.635	6.707	
DEPTH CUT - / FILL +	0	<i>19</i> .0-	-0.13	0.68	0.72	0.81	0.88	
EXISTING SURFACE LEVEL	5.77	5.78 5.78	5.79	5.8	5.81	5.82	5.83	
OFFSET FROM CENTRELINE	-17.84	-15.2 -14.6	-12.42	-7.42	-5.92	-2.42	0	

CH 1350

В	DESIGN UPDATE	NM	TH*	DP*	09.06.14	SCALE 1:200 AT ORIGINAL SIZE
A	PRELIMINARY	NM				
No	Revision Note: * indicates signatures on original issue of drawing or last revision of drawing	Drawn	Job Manager	Project Director	Date	
Plot	Date: 9 June 2014 - 4:54 p.m. Plotted by: Nicki Mackie	Ca	d File No:	G:\51\3146	B Whakatu\	CADD\Drawings\51-31468-C251_C258.dwg

1 in 4 1 in 4

5.471 5.09 5.09 5.447

-0 -0.38 -0.38

5.47 5.47 5.47 5.48

0.79

5.49 5.49

4.738 6.43 6.525 6.42 6.375 5.541 -0.83 1.01 0.89 0.01 5.57 5.52 5.53 5.53

		1 in 4	1 in -6	_	-3%		-3%	=	1 in -6	1 in -4	-1 in 4	
DATUM R.L. 3.00		Π										
DESIGN SURFACE LEVEL	5.394	5.023 5.023 5.33	6.164	6.209	6.314	6.385	6.28	6.235	5.401	4.641	5 507	
DEPTH CUT - / FILL +	0	-0.38 -0.38 -0.08	0.74	0.78	0.87	0.94	0.82	0.78	20.0-	-0.84		~
EXISTING SURFACE LEVEL	5.39	5.4 5.4 5.41	5.43	5.43	5.44	5.45	5.46	5.46	5.47	5.49 5.40	5.5	
OFFSET FROM CENTRELINE	-15.68	-14.2 -13.6 -12.37	-7.37	-5.87	-2.37	0	3.5	5	10	13.04	17.1	

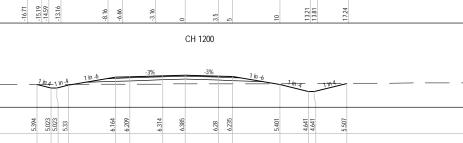
0.92

5.51

CH 1165

HASTINGS DISTRICT COUNCIL GHD 134 Queen Street East Hastings 4156, New Zealand T 64 6 870 9105 F 64 6 873 5281 W www.ghd.com

DO NOT SCALE	Drawn N. MACKIE	Designer N. DEVERY
GHD Limited	Drafting Check	Design Check
This document may only be used by GHD's client (and any other person who	Approved (Project Director)	
GHD has agreed can use this document)	Date	
for the purpose for which it was prepared and must not be used by any other person or for any other purpose.	Scale 1:200	This Drawing must not be used for Construction unless signed as Approved



					CH 1250						
1 in -6	-		-3%		-3%		1 in -6				
	-							1 in .4	Π	1 in 4	1
	6.28	6.325	643	6.525	6.42	6.375	5.541	4 738	4.738	5.595	

DATUM R.L. 3.00 DESIGN SURFACE LEVEL

DEPTH CUT - / FILL +

EXISTING SURFACE LEVEL

OFFSET FROM CENTRELINE

Plot Date: 9 June 2014 - 4:54 p.m. Plotted by: Nicki Mackie

DATUM R.L. 3.00 DESIGN SURFACE LEVEL

DEPTH CUT - / FILL +

EXISTING SURFACE LEVEL

OFF SET FROM CENTRELINE

5.77	5.78 5.78	5.79	0	5.81
-17.84	-15.2	-12.42	C 4	E 07

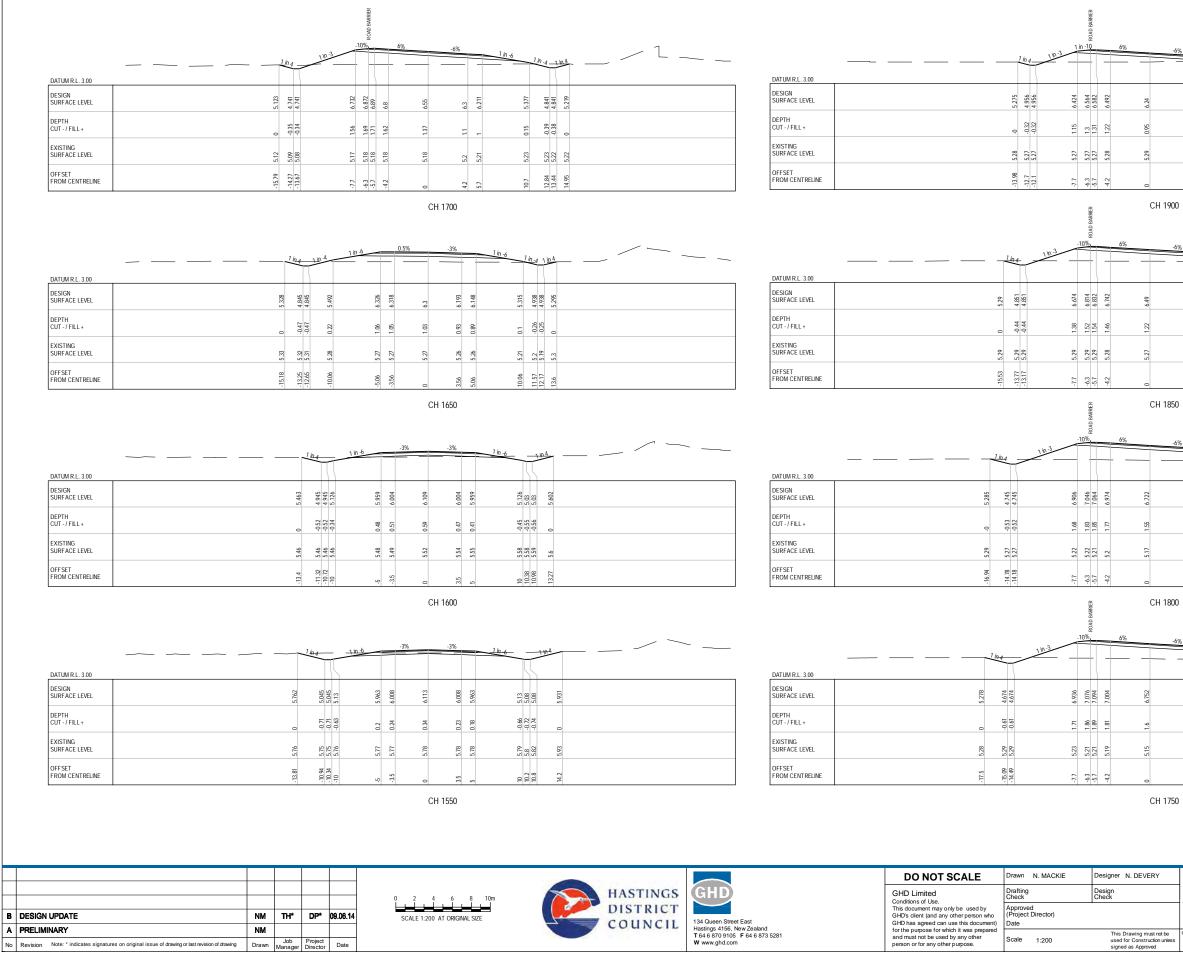
%		1 in -6		_	1 in 4	 	~_	
6.153	6.108	5.274	5.13	5.13	6.056			
0.17	0.13	-0.71	-0.85	-0.85	9			
5.98	5.98	5.98	5.98	5.98	6.06			
3.5	5	P	10.58	11.18	14.88			

CH 1500

67

-3%		1 in -6	1 in .4 -		1 in 4		 -1	 _	
			11-4	Π					
6.602	6.557	5.724	5.037	5.037	5.92				
0.76	0.71	-01	-0.82	-0.83	0				
5.84	5.84	5.84	5.86	5.87	5.92				
3.5	5	0	12.75	13.35	16.88				

			PRELIMINARY
Client	HASTINGS	DISTRICT COUNCIL	
Project		ARTERIAL PROJECT	
Title	CROSS SE	CTIONS - ROAD01	
	CH 1165 TC	CH 1500	
Original Size	Drawing No:	51-31468-C25	4 Rev: B



Plot Date: 9 June 2014 - 4:55 p.m. Plotted by: Nicki Mackie Cad File No: G:\51\31468 Whakatu\CADD\Drawings\51-31468-C251_C258.dwg

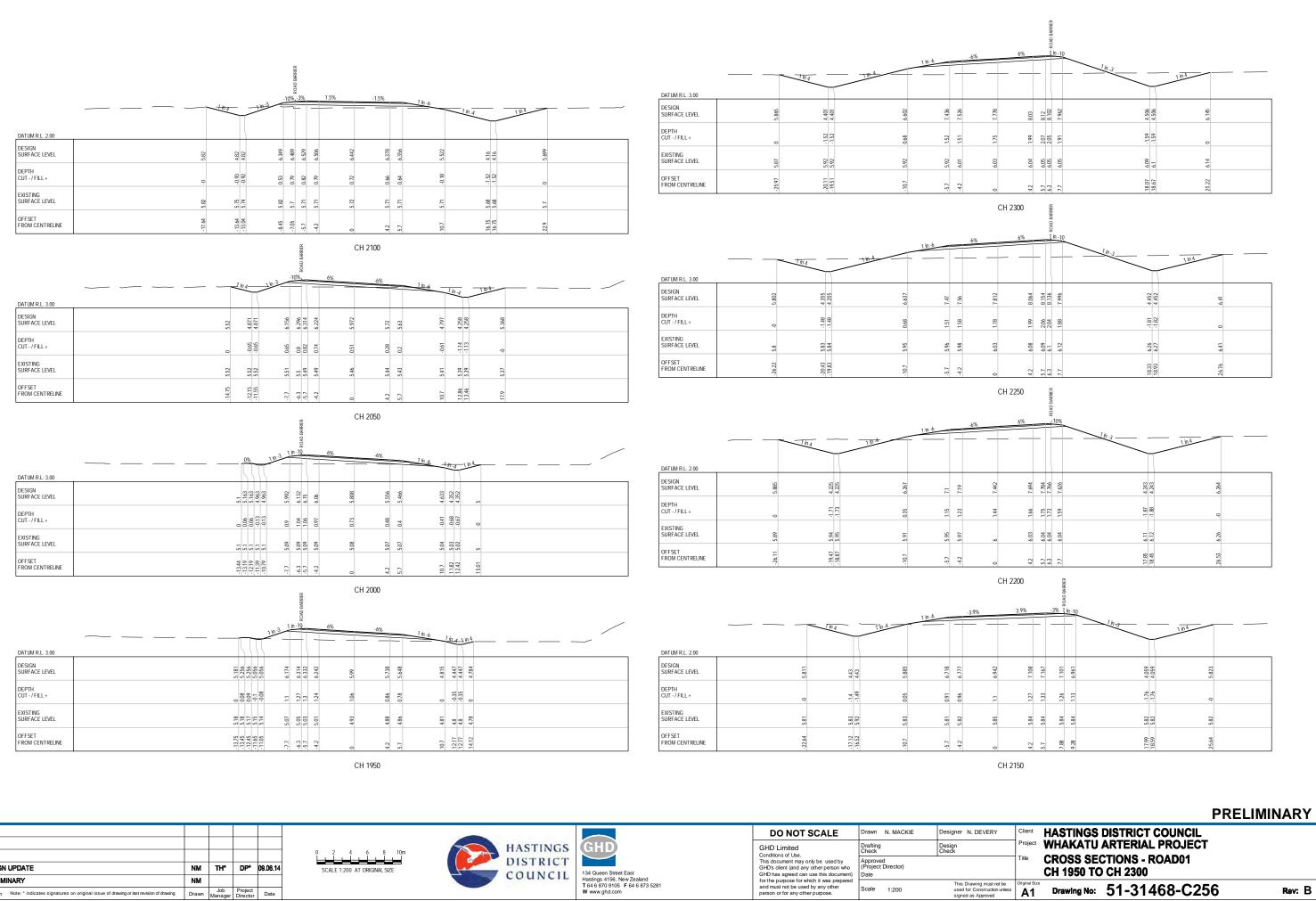
	=	1 in -6	<u>1 in .</u>	1	-1 in 4	
5.988	5.898	5.065	4.742	4.742	5.35	
89.0	0.58	-0.27	-0.6	-0.6	0	
5.31	5.31	5.33	5.34	5.34	5.35	
4.2	5.7	10.7	11.99	12.59	15.03	

-6%		1 in -6	<u>1 in -4</u>		in 4	
6.238	6.148		5.315 4.715	4.715	5.101	
-	0.92		0.15	-0.41	0	
5.24	5.23	:	5.16 5.13	5.12	5.1	
4.2	5.7			13.7	15.24	

-6%	_	1 in .6	<u>1 in -4</u>	1	in 4	
6.47	6.38	5.547	4.668	4.668	5.017	
1.32	1.24	0.47	-0.37	-0.36	ņ	
5.15	5.14	5.08	5.03	5.03	5.02	
4.2	5.7	10.7	14.21	14.81	16.21	

-6%		1 in -6					/	 <u> </u>	
			<u>1 in -4</u>		in 4	_			-
6.5	6.41	5.577	4.614	4.614	5.082				
1.39	1.3	0.48	-0.47	-0.47	ņ				
5.11	5.11	5.1	5.09	5.09	5.08				
4.2	5.7	10.7	14.55	15.15	17.02				

			PRELIMINARY
Client	HASTINGS	DISTRICT COUNCIL	
Project	WHAKATU	ARTERIAL PROJECT	
Title	CROSS SEC	CTIONS - ROAD01	
	CH 1550 TC	CH 1900	
Original Size	Drawing No:	51-31468-C25	5 Rev: B



134 Queen Street East Hastings 4156, New Zealand T 64 6 870 9105 F 64 6 873 5281 W www.ghd.com

Date

Scale 1:200

COUNCIL

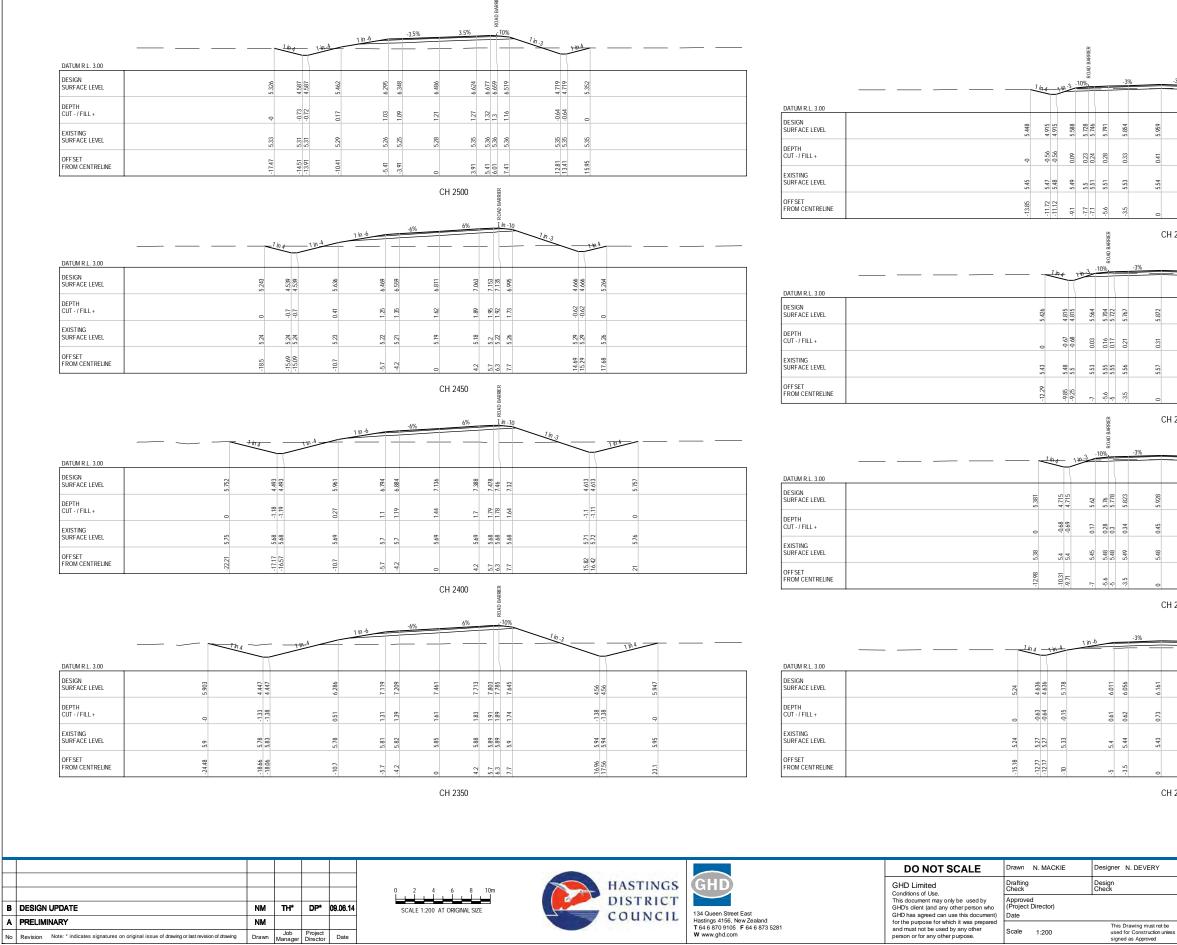
- 1						
	B	DESIGN UPDATE	NM	TH*	DP*	09.06.14
	A	PRELIMINARY	NM			
Ī	No	Revision Note: * indicates signatures on original issue of drawing or last revision of drawing	Drawn	Job Manager	Project Director	Date

Job Project Manager Director Date Cad File No: G:\51\31468 Whakatu\CADD\Drawings\51-31468-C251_C258.dwg

SCALE 1:200 AT ORIGINAL SIZE

Plot Date: 9 June 2014 - 4:55 p.m. Plotted by: Nicki Mackie

CROSS SECTIONS - ROAD01 Title CH 1950 TO CH 2300 This Drawing must not be used for Construction unless signed as Approved A1 Drawing No: 51-31468-C256 Rev: B



No Revision Note: * indicates signatures on original issue of drawing or last revision of drawing Job Project Manager Director Date Plot Date: 9 June 2014 - 4:55 p.m. Plotted by: Nicki Mackie Cad File No: G:\51\31468 Whakatu\CADD\Drawings\51-31468-C251_C258.dwg

Drawn

Scale 1:200

35 555 031 417 554 0.29 567 554 0.23 627 553 0.1 94 553 0.47 10 552 0.47 11/85 5.52 0.47	
35 555 0.31 4.17 554 0.29 5.67 5.54 0.25 6.27 5.54 0.25 6.27 5.53 0.1 9.4 5.53 0.47 10 5.52 0.47 11.08 5.52 0	
35 555 0.31 4.17 554 0.29 5.67 5.54 0.25 6.27 5.54 0.25 6.27 5.53 0.1 9.4 5.53 0.47 10 5.52 0.47 11.08 5.52 0	
35 555 37 555 547 554 627 554 9A 553 10 552 11.88 552	
3.5 5.55 4.17 5.54 5.67 5.54 7.67 5.53 1.627 5.53 1.627 5.53 1.1.68 5.53 1.1.68 5.52 1.1.68 5.52	
35 567 627 167 10 1186	
5. 5. 107 5. 722 5. 704 4. 922 5. 56 5. 56 5. 56	
0 02 0 0 01 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	
557 0 556 556 556 556 556 4 4 4 4 4 4 4 4 4 4	
CH 2650 11 10 10 10 10 10 10 10 10 10 10 10 10	
-3% <u>10 10 7 h-3</u> 10 4	
5 823 5 778 5 .778 5 .60 5 .478 5 .478	
86 662 662	
6	
CH 2600 법을 참 없 것 같은 것	
100	
2 90 5 99 5	
0.66 6.094 0.63 6.094 0.63 6.047 0.43 5.907 0.48 5.907 0.66 4.769 -0.66 4.769 5.431	

CH 2550

Title

PRELIMINARY HASTINGS DISTRICT COUNCIL Client WHAKATU ARTERIAL PROJECT **CROSS SECTIONS - ROAD01** CH 2350 TO CH 2700 A1 Drawing No: 51-31468-C257

Rev: B

		and the second se
DATUM R.L. 4.00	<u>1in.45 1in.66 .9% .21% .58% .4</u>	45% -2.2% -8.9% 1/n -3.3 1/n -3.4
DESIGN SURFACE LEVEL	6.399 6.399 6.399 6.399 6.965 6.966 6.966 7.143 7.285	7.058 6.831 6.831 6.831 6.831 6.831 6.831 6.831 6.831 6.831 6.337 5.533 5.533
DEPTH CUT - / FILL +	-0 0.61 0.61 1.24 1.23 1.23 1.23 1.23 1.23	1.33 1.18 1.18 1.25 1.25 1.25 0.33 0.33 0.01
EXISTING SURFACE LEVEL	5.85 5.79 5.79 5.79 5.73 5.73 5.73 5.73 5.73 5.73 5.78	23 29 29 29 29 29 29 29 29 29 29 29 29 29
OFFSET FROM CENTRELINE		503 115.36 115.36 115.36 115.36 115.37 115.37 115.37 115.32 115.32 115.32 115.32 115.32 115.32 125.32 22.23 22.23 22.23 22.23 22.23 22.23 22.23 22.23 22.23 22.33 22.23 22.33 22.33 22.33 22.33 22.33 22.33 22.33 22.33 22.33 22.33 22.33 22.33 22.33 22.33 22.34 115.36 12.36 12.36 12.36 12.36 12.36 12.36 12.37
	на СН 28 902	للام الله الله الله الله الله الله الله
	1 n 31 in 10	340 He had be ha
DATUM R.L. 4.00		
DESIGN SURFACE LEVEL	5.451 5.756 6.476 0.4456 0.4456 0.4456 0.4456 0.4456 0.4456 0.4457 0.4427 0.4427 0.4427 0.4427	6.547 6.446 6.446 6.243 5.525 5.455 5.455 5.455 5.455 5.455
DEPTH CUT - / FILL +	0 0.31 0.87 1.13 1.1 1.1 1.1 1.1 1.1 1.1 1.1 1.1 1.	1 109 0.9% 0.03 0.03 0.03 0.03
EXISTING SURFACE LEVEL	6.65 6.45 6.43 6.43 6.43 6.43 6.43 6.43 6.43 6.43	5 45 1
OFFSET	11326 11326 11326 1107 1107 1107 1107 1107 1107 1107 110	35 687 1037 1037 15,23 15,23 15,26 15,26 15,26

									DO NOT SCALE	Drawn N. MACKIE	Designer N. DEVER
						0 2 4 6 8 10m	HASTINGS	GHD	GHD Limited Conditions of Use.	Drafting Check	Design Check
В	DESIGN UPDATE	NM	TH*	DP*	09.06.14	SCALE 1:200 AT ORIGINAL SIZE	DISTRICT	134 Queen Street East	This document may only be used by GHD's client (and any other person who GHD has agreed can use this document)	Approved (Project Director)	
A	PRELIMINARY	NM					COUNCIL	Hastings 4156, New Zealand T 64 6 870 9105 F 64 6 873 5281	for the purpose for which it was prepared and must not be used by any other	Scale 1:200	This Drawing must
No	Revision Note: * indicates signatures on original issue of drawing or last revision of drawing	Drawn	Job Manager	Project Director	Date			W www.ghd.com	person or for any other purpose.	1:200	used for Construction signed as Approved

PRELIMINARY

Y	Client	HASTINGS	DISTRICT COUNCIL	
	Project	WHAKATU	ARTERIAL PROJECT	
	Title		CTIONS - ROAD01	
		CH 2750 TC) CH 2840	
not be on unless	Original Size	Drawing No:	51-31468-C258	Rev: B

DATUM R.L. 4.00		l in 4	1 in .4	in -6	3%	-3%		<u>1 in -6</u>	1 h.4	
DESIGN SURFACE LEVEL	6.19	5.816 5.816	6.958	7.791 7.746	7.641	7.536	7.491 7.491 7.556 7.456	906	5.478 5.478 5.718	
DEPTH CUT - / FILL +	ę	-0.45 -0.45	0.5	1.67	1.9	1.83	1.78 1.78 1.84 1.72	1.15	-0.31	
EXISTING SURFACE LEVEL	6.19	6.26 6.26	6.46	6.12 5.96	5.75	5.71	5.71 5.71 5.72 5.73	5.76	5.78 5.76 5.72	
OFFSET FROM CENTRELINE	-16.67	-15.17 -14.57	-10	-5	0	3.5	5 5.35 6.45	9.75	15.47 16.07 17.03	

CH 150

CH 100

DATUM R.L. 4.00	<u>0% 1m4</u>	1 in -63	<u> </u>	<u>3% 1 in 6</u> 1	h-4 tin A	
DESIGN SURFACE LEVEL		7.867	717.7	7.612 7.567 7.557 7.532 7.532 6.982	5.575 5.575 5.89	
DEPTH CUT - / FILL +	0 002 -0.17 -0.17	0.09 1.28 1.24	1.31	1.69 1.68 1.74 1.74 1.65	-0.44 -0.39 0	
EXISTING SURFACE LEVEL	6.1 6.1 6.09 6.09 6.01	6.58 6.58	6.41	5.92 5.89 5.89 5.88 5.88 5.88 5.94	6.02 5.97 5.89	
OFFSET FROM CENTRELINE	-16.93 -16.86 -15.86 -15.86 -14.46	-10	0	3.5 5 5.35 6.45 9.75	15.38 15.98 17.24	

	<u>1in.4 1in.6</u>	-10%	3.2% 3%	-10% 1 in -6	<u>1 in .4</u> 1.4%	
DATUM R.L. 3.00		5				
DESIGN SURFACE LEVEL		6.505 6.605 6.54 6.54	6.83 6.823 6.928	7.033 7.078 7.078 7.043 7.043 6.493		
DEPTH CUT - / FILL +	-0 -0.01 0.33 0.33	0.97 0.97 0.97	<u>1.39</u> 1.53	1.65 1.7 1.76 1.68		
EXISTING SURFACE LEVEL	5.78 5.65 5.65 5.62 5.62	5.61 5.58 5.57 5.57	5.44 5.44 5.44	5.38 5.38 5.38 5.38 5.38 5.38 5.38 5.38	5.07	
OFFSET FROM CENTRELINE	-17.15 -17.02 -16.22 -16.22 -14	-10.7 -9.6 -9.25 -9.25	-0.14 0 3.5	7 8.5 8.5 9.95 13.25	19.51 22.53	

	 1 in 4	1 in -6 3%	-3% <u>1 in -6</u>	1 in -4 1 in 4	
DATUM R.L. 4.00					
DESIGN SURFACE LEVEL	6.254 6.02 6.02	6.727 7.56 7.515	7.41 7.305 7.26 7.25 7.25 7.25 7.25 7.25 7.25 6.675	5. <i>6</i> 73 5. <i>6</i> 73 6.16	
DEPTH CUT - / FILL +	0 -0.22 -0.24	0.54	0.69 0.82 0.87 0.087 1.07 1.07 0.61	-0.48 -0.54	
EXISTING SURFACE LEVEL	6.25 6.24 6.26	6.18 6.62 6.65	6.73 6.48 6.39 6.39 6.39 6.16 6.16	6.16 6.21 6.16	
OFFSET FROM CENTRELINE	-14.37 -13.43 -12.83	-10	0 5 5 6.45 6.45 9.75	13.76	

		1 in -6		3%			-10% 1 in		<u>n-4</u>	<u>in4</u>
DESIGN SURFACE LEVEL	5.629 5.818 5.818 5.818 5.618 5.618 5.618	6.564	6.609	6.691	6.901	6.974	7.019 7.019 7.084 6.984	6.434	5.058 5.058	5.461
DEPTH CUT - / FILL +	0 002 014 014	1.03	1.08	1.18	1.35	1.49	1.47 1.47 1.52 1.41	1.04	-0.37	Q
EXISTING SURFACE LEVEL	5.63 5.63 5.63 5.59 5.59 5.59	5.54	5.52	5.51	5.55	5.48	5.55 5.55 5.57 5.58	5.4	5.43 5.44	5.46
OFFSET FROM CENTRELINE	-16.33 -16.57 -15.57 -13.77 -13.77 -13.77	-7.72	-6.22	-3.5	3.5	5.92	7.42 7.42 7.77 8.87	12.17	17.67 18.27	19.88

	$1 \text{ in -6} - \frac{-12\%}{3\%} - \frac{-10\%}{10\%} \frac{1}{10.6}$
DATUM R.L. 4.00	<u>1 in 6 ·12% ·3% ·10% 1 in 6 1 in 4 · 1 in 8</u>
DESIGN SURFACE LEVEL	6.6882 6.6882 6.985 6.985 6.985 6.985 6.985 6.985 6.985 6.985 6.993 6.643 6.643 6.643 6.643 6.643 6.643 6.643 6.643 6.643 6.643 6.645 7.722 6.642 6.645 7.722 6.645 7.722 6.645 6.645 7.722 6.645 6.722 6.645 6.65
DEPTH CUT - / FILL +	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
EXISTING SURFACE LEVEL	6.689 6.73 6.73 6.73 6.73 6.73 6.73 6.73 6.73
OFFSET FROM CENTRELINE	

CH 50

						0 2 4 6 8 10m
В	DESIGN UPDATE	NM	TH*	DP*	09.06.14	SCALE 1:200 AT ORIGINAL SIZE
A	PRELIMINARY	NM				
No	Revision Note: * indicates signatures on original issue of drawing or last revision of drawing	Drawn	Job Manager	Project Director	Date	
Plot	Date: 9 June 2014 - 5:18 PM Plotted by: Nigel Devery	Ca	d File No:	N:\NZ\Chris	stchurch\Pro	ojects\51\31468 Whakatu\CADD\Drawings\51-31468-C259.dwg



DO NOT SCALE	Drawn N. MACKIE	Designer N. DEVERY		HASTINGS DISTRICT COUNCIL
GHD Limited Conditions of Use.	Drafting Check	Design Check		WHAKATU ARTERIAL PROJECT
This document may only be used by	Approved (Project Director) Date		Title	CROSS SECTIONS - ROAD10 CH 50 TO CH 350
for the purpose for which it was prepared and must not be used by any other person or for any other purpose.	Scale 1:200	This Drawing must not be used for Construction unless signed as Approved	Original Size	Drawing No: 51-31468-C259 Rev: B

PRELIMINARY

СН	300

CH 350

	STREAM OF CONTRACT OF CONTRACT.
	<u>1 In A 1 In 6 -2.7% -3% -10% 7 h.3</u>
DATUM R.L. 3.00	
DESIGN SURFACE LEVEL	5 5 156 5 691 6 311 5 687 6 512 5 682 5 588 6 512 5 588 6 512 5 588 5 585 5 588 5 585 5 585 5 585 5 585 5 585 5 585 5 585 5 585 5 585 5 585 5 585 5 58
DEPTH CUT - / FILL +	0 049 001 001 001 001 001 001 001 001 001 00
EXISTING SURFACE LEVEL	
OFFSET FROM CENTRELINE	
	CH 400
	2 20/ 2 29/ 20/
	<u>1h.4</u> <u>1h.6</u> <u>3276</u> <u>378</u> <u>105</u> <u>107</u> <u>10</u>
ATUM R.L. 3.00	
ESIGN URFACE LEVEL	4.785 6.062 6.012 6.013 6.136 6.137 6.138 6.138 6.138 6.138 6.138 6.138 6.138 6.138 6.138 6.138
EPTH UT - / FILL +	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
XISTING URFACE LEVEL	
DFFSET	2 2 2 4 4 2 2 2 5 5 4 2 2 2 5 5 5 4 2 3 3 5 6 9 9 3
FROM CENTRELINE	
	CH 350 ਸ਼ੁੱ
	CH 350 gg
	<u>110-4</u> <u></u>
DATUM R.L. 3.00	
IESIGN URFACE LEVEL	4 4 23 5 5 208 5 5 8 24 5 5 8 24 4 7 75 5 5 7 9 20 5 5 8 24 4 7 75 5 5 7 9 20 5 7 9 7 9 7 9 7 9 7 9 7 9 7 9 7 9 7 9 7
iepth IUT - / Fill +	0.038 0.038 0.038 0.038 0.038 0.038 0.037 0.037 0.037 0.037 0.037 0.037 0.037 0.037 0.037 0.037 0.037 0.037 0.037 0.037 0.037 0.038 0.
XISTING URFACE LEVEL	
DFFSET	
FROM CENTRELINE	
	CH 300 ਸ਼ਿੱਲ ਰੋਟ
	<u>1 in 6 -10% -3% 2% In 10 1/n-3</u>
DATUM R.L. 2.00	
DESIGN SURFACE LEVEL	4 466 5 512 5 512 5 512 5 512 5 512 5 512 5 517 5 517 5 517 5175 5175
IEPTH IUT - / FILL +	
JI-/FILL+	-0 0 0 0 0 0 12

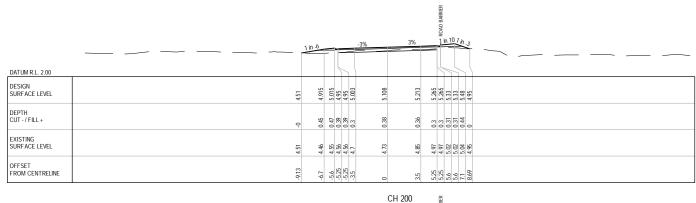
	A STATE AND A STATE AN
	10.4 10.6 -2.7% -3% -10% 1/p.3
DATUM R.L. 3.00	
DESIGN SURFACE LEVEL	5 5 15 6 6 241 5 6 041 6 241 6 512 6 512 6 512 5 598 5 598
DEPTH CUT - / FILL +	
EXISTING SURFACE LEVEL	
OFFSET FROM CENTRELINE	
	CH 400
	1n.4 1n.6 -3.2% -3.2% -3% -2.10% 1/n.3
DESIGN SURFACE LEVEL	5 5 10 8 8 9 11 12 12 12 12 12 12 12 12 12 12 12 12
DEPTH CUT - / FILL +	
EXISTING SURFACE LEVEL	
OFFSET	67 478 67 478 68 519 68 531 69 549 611 549 611 549 611 513 611 513 611 513 611 513 611 513 611 513 611 513 611 513 611 513 611 513 611 513 611 513 611 513 611 513 611 513 611 513 611 513 611 513 611 513
FROM CENTRELINE	$\begin{array}{c} -13.67\\ -13.67\\ -10.76\\ -5.36\\ -5.35\\ -5.36\\ -6.03\\$
	CH 350
	-3.2% $-3%$ $-3%$ $-10%$ 7.
DATUM R.L. 3.00	
DESIGN SURFACE LEVEL	4 4/2 4 4/2 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5
DEPTH CUT - / FILL +	
EXISTING SURFACE LEVEL	
OFFSET	1 1 4
FROM CENTRELINE	$\begin{array}{c c} -13.14 \\ \hline57 \\56 \\57 \\5$
	CH 300
	1 <u>in 6 -10% -3% 2% Lin -10</u> 1/0 -3
~ ·	
DATUM R.L. 2.00 DESIGN SURFACE LEVEL	2 2 32822 2 3 2 3282 × ->e
DEPTH	4 66 4 771 4 777 5 32 5 335 5 3407 5 5 51 5 561 5 561 5 561 5 562 5 563 </td
CUT - / FILL +	0 0 0 0 0 0 0 0 0 0 0 0 0

SURFACE LEVEL	4.785	5.512	6.062	074.7
DEPTH CUT - / FILL +	Ŷ	0.64	0.88	
EXISTING SURFACE LEVEL	4.78	4.88	5.19	
OFFSET FROM CENTRELINE	-13.67	-10.76	-7.46	101
1				
			h	
			1 in -6	_

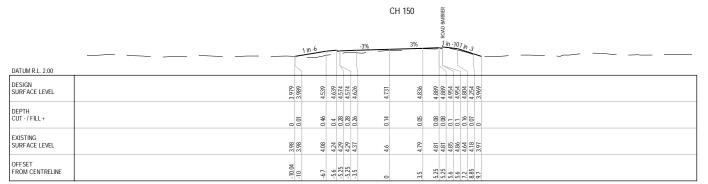
	28E R	
	ROAD BARRER	
	1n.4 1n.6 -2.7% -3% -10% 1/h.3	
DATUM R.L. 3.00		
DESIGN SURFACE LEVEL	07 12 18 23 25 18 23 25 25 25 25 25 25 25 25 25 25 25 25 25	
DEPTH	5,156 5,691 5,691 6,241 6,241 6,225 6,225 6,225 6,225 6,225 6,225 6,225 6,225 6,225 6,225 6,225 6,225 6,225 6,225 6,225 6,225 6,225 6,225 6,225 6,221 6,225 6,221 6,225 6,221 6,225 6,221 6,222	
CUT - / FILL +	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	
EXISTING SURFACE LEVEL	516 516 543 553 553 553 553 553 553 553 553 553	
OFFSET FROM CENTRELINE		
I		
	CH 400 HE HAR AND	
	10.4 10.6 -32% -32% -3% -10% 7 h.3	
DATUM R.L. 3.00		
DESIGN SURFACE LEVEL	4,185 5,512 6,602 6,097 6,612 6,073 6,073 6,073 6,073 6,073 6,073 5,108 5,518 6,173 5,518 6,5288 6,528 6,528 6,528 6,528 6,528 6,528 6,528 6,528 6,528 6,528	
DEPTH		
CUT - / FILL + EXISTING	0 0 0 0 0 0 0 0 0 0 0 0 0 0	
SURFACE LEVEL	4 78 4 78 5 5 5 5 5 7 5 4 9 4 9 5 1 1 1 5 1 1 1 1 5 1 1 1 1 1 5 1 1 1 1 1 5 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
OFFSET FROM CENTRELINE	-13.67 -10.76 -10.76 -6.13 -6.13 -6.13 -6.13 -6.13 -6.13 -6.13 -6.13 -6.13 -6.13 -6.13 -6.13 -6.13 -6.13 -6.13 -6.13 -7.48 -7.48 -7.48 -6.13 -7.48 -7.49 -7.48 -7.49 -7.48 -7.48 -7.49 -7.48 -7.48 -7.48 -7.48 -7.48 -7.48 -7.48 -7.	
	CH 350 뗥	
	CH 350	
	11n-4 11n-6	
DATUM R.L. 3.00		
DESIGN SURFACE LEVEL	4 4 4 23 5 5 7 26 5 5 8 8 7 5 5 8 8 7 6 1 8 4 7 7 7 5 5 8 8 7 5 5 8 8 7 5 5 8 8 7 5 5 8 8 7 6 7 8 8 7 7 8 8 7 7 8 7 8 7	
DEPTH CUT - / FILL +	0 0 0 0 0 0 0 0 0 0 0 0 0 0	
EXISTING SURFACE LEVEL		
OFFSET	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	
FROM CENTRELINE		
	CH 300	
	ADE	
	200 200 <u>200</u> <u>1</u> in 10	
	<u>1 in 6 -10% -3% 2% <u>Jin 10</u> 1/n.3</u>	
DATUM R.L. 2.00 DESIGN SURFACE LEVEL	200 200 <u>200</u> <u>1</u> in 10	

	Ě	
	83	
	<u>10.4 10.6 21.8 300 10.3</u>	
DATUM R.L. 3.00		
DESIGN SURFACE LEVEL	5 156 5 6 12 6 6 12 6 5	
DEPTH CUT - / FILL +	0 0.01 0.01 0.01 0.04 0.04 0.05 0.05 0.05 0.05 0.04 0.04	
EXISTING SURFACE LEVEL	5.16 5.43 5.53 5.57 5.57 5.57 5.57 5.57 5.57 5.5	
DFFSET FROM CENTRELINE	-15.64 -13.5 -13.5 -1.10.2 -1.10.2 -1.10.2 -1.13.5 -1.	
I		
	CH 400	
	<u>11n.4</u> <u>11n.5</u> <u>-32%</u> <u>-3%</u> <u>-10%</u> <u>1/h.3</u>	
DATUM R.L. 3.00		
DESIGN SURFACE LEVEL	4.785 5.512 5.612 6.062 6.0912 6.138 6.138 6.138 5.538 5.538 5.538 5.538 5.538 5.538 5.538 5.5438	
DEPTH CUT - / FILL +	0 0 0 0 0 0 0 0 0 0 0 0 0 0	
EXISTING SURFACE LEVEL	4 78 5 519 5 519 5 513 5 513 5 514 5 515 5 515 5155 51	
DFFSET FROM CENTRELINE	-13.67 -10.76 -10.76 -1.7.46 -6.36 -6.01 -7.13 -7.14 -	
	CH 350	
	$1n4$ $1n6$ -32% -3% -10% $1h_{,3}$	
DATUM R.L. 3.00		
DESIGN	10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4 10.4	
DESIGN SURFACE LEVEL	0 4423 0 0 0 0 0 0 0 0 0 0 0 0 0	
DESIGN SURFACE LEVEL DEPTH CUT - / FILL + EXISTING	4.42 0 4.42 4.42 4.7 0 5.158 4.423 4.7 0.51 5.238 4.423 5 0.79 5.158 5.238 4.9 0.87 5.942 4.43 4.9 0.87 5.942 4.43 4.9 0.87 5.867 5.849 4.9 0.887 5.887 5.849 4.9 0.89 5.887 5.849 4.9 0.99 5.393 5.849 4.9 0.93 5.849 5.849 4.17 0 3.549 5.849 4.17 0 3.549 5.942	
DESIGN SURFACE LEVEL DEPTH CUT - / FILL + EXISTING SURFACE LEVEL DFFSET	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	
DESIGN SURFACE LEVEL DEPTH CUT - / FILL + EXISTING SURFACE LEVEL DFFSET	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	
DESIGN SURFACE LEVEL DEPTH CUT - / FILL + EXISTING SURFACE LEVEL DFFSET	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	
DESIGN SURFACE LEVEL DEPTH CUT - / FILL + EXISTING SURFACE LEVEL DFFSET	CH 300 CH 300	
DESIGN SURFACE LEVEL DEPTH CUT - / FILL + EXISTING SURFACE LEVEL DFFSET FROM CENTRELINE	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	
DESIGN SURFACE LEVEL DEPTH UT - / FILL + EXISTING SURFACE LEVEL DFFSET ROM CENTRELINE DATUM RL 2.00 DESIGN	100 100 100 100 611 100 100 100 612 100 100 100 613 100 100 100 614 100 100 100 615 100 100 100 615 100 100 100 615 100 100 100 615 100 100 615 100 100 615 100 100 615 100 100 615 100 100 615 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100<	
DESIGN SURFACE LEVEL DEPTH UT - / FILL + EXISTING SURFACE LEVEL DFFSET ROM CENTRELINE DATUM RL 2.00 DESIGN SURFACE LEVEL DEPTH	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	

								DO NOT SCALE	Drawn N. MACKIE
					0 2 4 6 8 10m	HASTINGS	GHD	GHD Limited	Drafting Check
						DISTRICT		This document may only be used by	Approved
	NM	TH*	DP*	09.06.14	SCALE 1:200 AT ORIGINAL SIZE	COUNCIL	134 Queen Street East	GHD's client (and any other person who GHD has agreed can use this document)	(Project Director)
	NM					COUNCIL	Hastings 4156, New Zealand T 64 6 870 9105 F 64 6 873 5281	for the purpose for which it was prepared	
s signatures on original issue of drawing or last revision of drawing	Drawn	Job Manager	Project Director	Date			W www.ghd.com	and must not be used by any other person or for any other purpose.	Scale 1:200
Plotted by: Nigel Devery	Ca	d File No:	N:\NZ\Chris	tchurch\Pro	ojects\51\31468 Whakatu\CADD\Drawings\51-31468-C260_C261.dwg				



							01120	0	RADDIE			
DAT UM R.L. 2.00		1 in -6	F	-		-3%		3%	PUAL PUAL	in 10	1 in .3	·
DESIGN SURFACE LEVEL	C C C C C C C C C C C C C C C C C C C	4.680	4.789	4.724	4.776	2	4.881	4.986	5.039	5.104	5.254 5.254 4.012	2 9
DEPTH CUT - / FILL +		053	0.54	0.39	0.25	1310	0.26	0.16	20.07	0.14	0.43	>
EXISTING SURFACE LEVEL	8	4 16	4.25	4.33	4.53	r;	4.62	4.82	4.97	4.96	4.83	
OFFSET FROM CENTRELINE		11.6-	-5.6	-5.25	-3.5	Ċ.	0	3.5	5.25 5.25	5.6	0.6 1.7	v 25



No Revision Note: * indicates signa Plot Date: 9 June 2014 - 5:20 PM Plotted by: Nigel Devery

B DESIGN UPDATE A PRELIMINARY

D DE

CH 250

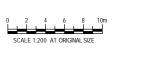
Design Check

PRELIMINARY HASTINGS DISTRICT COUNCIL WHAKATU ARTERIAL PROJECT Designer N. DEVERY Client CROSS SECTIONS - ROAD11 SHEET 1 Title This Drawing must not be used for Construction unless A1 Drawing No: 51-31468-C260 Rev: B

	1 n-5 - 3% - 3% - 3% - 3% - 10% 1/h,3	
DATUM R.L. 4.00		
DESIGN SURFACE LEVEL	6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	
DEPTH CUT - / FILL +	0 016 0.03 0.03 0.03 0.03 0.03 0.03 0.03 0.0	
EXISTING SURFACE LEVEL	ゆゆゆ ゆ ゆ ゆ ゆ ゆ ゆ ゆ ゆ ゆ ゆ ゆ ゆ ゆ ゆ ゆ ゆ ゆ	
DFFSET ROM CENTRELINE		
·	СН 700 🚆	

						01	1700		BARRIEI					
DATUM R.L. 4.00		1/104 1 in .4	1 in-6		3%		-3%		A ROAD	-10%	7	<u>n 3</u>	14	<u>n</u>
DESIGN SURFACE LEVEL	म ४	6.087	6.769 7.602	7.557	7.501	7.396	7.291	7.218	7.173	7.167	7232	6.482 6.482 6	9	6.531
DEPTH CUT - / FILL +	c	-0.24	0.2	0.54	0.34	0.41	0.48	0.57	0.62	0.62	0.7	-0.03	-0.53	0
EXISTING SURFACE LEVEL	4.4K.	6.42	6.57 6.98	7.01	7.16	6:99	6.81	6.65	6.55 6.55	6.55	6.53	6.51 6.53	6.53	653
OFFSET FROM CENTRELINE	¥	-15.21	-11.88	-5.38	-3.5	0	3.5	5.91	7.41	7.45	7.8	7.4 11.22 12.67	13.27	14.87

						0 2 4 6 8 10m
В	DESIGN UPDATE	NM	TH*	DP*	09.06.14	SCALE 1:200 AT ORIGINAL SIZE
A	PRELIMINARY	NM				
No	Revision Note: * indicates signatures on original issue of drawing or last revision of drawing	Drawn	Job Manager	Project Director	Date	
Plot	Date: 9 June 2014 - 5:20 PM Plotted by: Nigel Devery	Ca	d File No:	N:\NZ\Chris	tchurch\Pro	- ojects\51\31468 Whakatu\CADD\Drawings\51-31468-C260_C261.dwg





DO NOT SCALE	Drawn N. MACKIE	Designer N. DEVERY	1	HASTINGS DISTRICT COUNCIL
GHD Limited Conditions of Use.	Drafting Check	Design Check		WHAKATU ARTERIAL PROJECT
This document may only be used by GHD's client (and any other person who GHD has agreed can use this document)	Approved (Project Director) Date			CROSS SECTIONS - ROAD11 SHEET 2
for the purpose for which it was prepared and must not be used by any other person or for any other purpose.	Scale 1:200	This Drawing must not be used for Construction unless signed as Approved	Original Size	Drawing No: 51-31468-C261 Rev: B

PRELIMINARY

Appendix B – Lighting Subcategory Evaluation

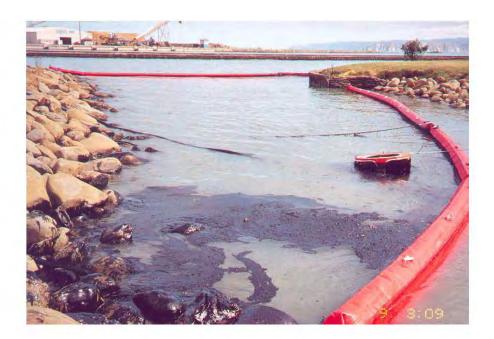
I	akatu Arterial Link, Hastings					6 JU	ne 201
	Parameter	Options		Weighting	Score	RE	SULT
	Vehicle Volume	Very High High Moderate Low Very Low	>20,000 12,000 to 25,000 6,500 to 15,000 3,000 to 7,500 <3,500	60 50 40 20 0	40	200 190 180	(>150)
	Road Designation (Lighting should support the road network hierarchy and encourage traffic to use the main routes which are designed to carry higher volumes of traffic)	Motorway/Free Major Arterial Arterial Collector Local	way	30 25 15 10 0	15	170 160 150 140	Ł
	Traffic Composition (A mix of motorised and non motorised traffic increases need for improved visibility)	Mixed with very Mixed Vehicles only	y high proportion of non-motorised A mix of motorised and non motorised traffic Very few cyclists or pedestrians present	30 10 0	10	130 120	
	Pedestrian and Cycle Volumes	Very High High Moderate	Major central city road Busy town centre or suburban road Urban cycle route or Rural where pedestrians/cyclists are present (eg school) Residential road	20 10 5 0	10	110 100	to 120)
	Speed Limit	Low Very High High Moderate Low	90 or 100 km/h 70 or 80 km/h 60 km/h 50 km/h or less	20 15 5 0	15	90 80 70	V3 (75
	Parked Vehicles (Increased number of parked vehicles on the road generates more pedestrian movements including road crossings)	Many Some Few - None	Main central city road Urban with some on-road parked vehicles or Rural with few on-road parked vehicles Urban road with few/no cars parked on road	10 5 0	5	60 50	
	Traffic Generation from Abutting Properties (High levels of traffic to and from abutting properties increases the number of potential conflicts)	High Moderate Low	Busy commercial area, supermarket, shops etc Some small businesses along road Traffic generated from residential properties/farms in neighbourhood	10 5 0	10	40 30	
	Ambient Luminance (Higher ambient luminance can reduce contrast and therefore higher levels of lighting are required to compensate)	Very High High Moderate	Central city shopping area creating high vertical illuminance Arterial road with small shopping centre Urban residential industrial area with minimal private lighting	20 15 5	5	20 10	2 (<25)
		Low	Rural /semi rural area, dark surrounds	0		0	

Appendix C – HBRC Spill Management Plan



HAWKE'S BAY REGIONAL COUNCIL

SPILL MANAGEMENT PLAN



Prepared by Ian Lilburn Hawke's Bay Regional Council August 2011

SPILL MANAGEMENT PLAN

Resource Management The Resource Management Act (RMA) sets out how we should manage our environment. It is based on the idea of sustainable management of our resources – or in other words, protecting the quality of our soil, air and water from being damaged beyond repair. The RMA isn't about stopping any activity that effects the environment. It is about undertaking activities in a manner that will have minimal impact to the environment. '*Every person has a duty to avoid, remedy, or mitigate any* adverse effect on the environment..' Section 17 of the RMA.

For any significant incident that could effect the environment, the steps taken should be:

- Cease all work in that area immediately and secure the site.
- Containment and control actions are to be employed as soon as possible.
- Call the project manager and let them know what has happened.
- Notify HBRC of incident 0800 108 838 (Pollution Hotline).
- A site manager should visit and inspect the site immediately, overseeing containment and control actions.
- Management and/or HBRC will issue authority to recommence work.
- An incident report be completed and submitted to the Regional Council.

Site management key factors

- The site manager/overseer is familiar with the resource consent conditions and this plan
- A copy of the consent and this Spill Management Plan is held on site
- All those working at the site are aware of their obligations and know what to do in the event of an incident
- A spill cleanup kit be available on site
- Regular 'tool-box' meetings are recommended to discuss site progress, safety, and environmental matters

Hazards and Controls:

ΑCTIVITY	RISK	ENVIRONMENTAL EFFECT	CONTROLS
REFUELLING	- Spillage - Wash-off - Fire	 Pollution of waterways, streams and storm-water systems Soil Contamination Ecological Damage (plant-life; wild-life) 	 Prevent spills: Inspect machines for any leaking fluids prior to starting job. Use established refuelling points Locate fuel tanks away from waterways Bunding of fuel tanks No hot refuelling Fire Prevention Plan Containment: Dig hole, create a bund, or use container to contain spill Stop the spill or leak, if safe to do so Create a barrier to keep out of waterway and contain Immediate Clean Up: Sawdust or suitable absorbent to soak up excess Scrape off affected topsoil and dump spoil in approved dumping-site only Contact HBRC pollution hotline 0800 108 838

ACTIVITY	RISK	ENVIRONMENTAL EFFECT	CONTROLS
WORKING NEAR/IN RIVERS, STREAMS, and COASTAL AREAS	-Machinery failure - Leakage / Spillage - Bank and or bed damage - Wash-off	 Pollution of waterways, streams and storm-water systems Soil Contamination Ecological Damage (plant-life; wild-life) 	 Work to resource consent conditions (copy to be kept on site) Work to contract specifications Store plant, stores and equipment in approved storage areas only and away from water-courses Contact HBRC pollution hotline 0800 108 838 in the event of a spill that may or will escape to water. Have a spill kit on hand Be familiar with what to do in the event of a spill or leak
CHEMICAL USE	- Leakage / Spillage - Wash off - Poisonous fumes - Explosion / Fire	 Short or Long-term contamination of water- ways, land and air Ecological poisoning Population poisoning through ingestion / inhalation 	 Abide by Material Safety Data Sheets (MSDS) for handling, storage and containment / clean-up information Emergency Plans Use effective and appropriate personal protective equipment (PPE) Contain and clean up, IF SAFE TO DO SO Contact HBRC pollution hotline 0800 108 838
DUMPING AND STORAGE OF MATERIAL, RUBBISH AND SPOIL	- Spillage - Wash-off - Fire - Rodent / Insect infestations - Blocked water-ways	 Pollution of waterways, streams and storm-water systems Soil Contamination Ecological Damage (plant-life; wild-life) Smell 	 Work to resource consent conditions (copy to be kept on site) Containment to prevent spread / wash-off Restricted access Waste material sites planned and managed Planned cartage and dumping for specific waste / spoil (including soil or waste contaminated from fuel, oils, human & animal waste, excess concrete HSNO) Proper rubbish disposal (skip bin, 200L drum etc)

INFORMATION SHEET ON ENVIRONMENTAL MATTERS.

ACTIVITY	RISK	ENVIRONMENTAL EFFECT	CONTROLS
DUST	- Reduced visibility - Air irritation - Company Image	 Personal –irritation, stress Amenity/aesthetics Crop damage 	 Dampen down tracks and areas of loose spoil Management arrange for mailbox drop if necessary Restrict hours of work Restricted vehicle movement and speed Designated park-up areas Use effective and appropriate PPE
NOISE	 Excessive noise Noise vibration Company Image 	 Personal irritation and stress Disruption to wild-life 	 Restrict vehicle, plant and equipment revs Baffles and muffling Restrict hours of work Management arrange for mailbox drop if necessary Use effective and appropriate PPE
EARTH-WORKS	- Undermining - Destabilisation - Flooding - Silt runoff	 Pollution of waterways, streams and storm-water systems Soil Contamination Ecological Damage (plant-life; wild-life) Erosion Silt Build-up / flooding 	 Work to resource consent conditions Use erosion and sediment controls as per HBRC guidelines, and as per plans and project methodology Work to boundaries in contract specifications Water pumps – water diversion Control storm-water and surface water run-off Daily site checks Restricted access / Barriers Stabilise surfaces as soon as practical
SITES OF NATURAL, HISTORICAL, AND CULTURAL SIGNIFICANCE (e.g. birds, wetlands, old pa sites, tapu sites, bodily remains etc)	 Desecration of burial sites Destruction of artefacts Disruption of wild life breeding sites Destruction of rare breeds of fauna and flora Company Image 	 Ecological Impact (plant- life; wild-life) Cultural offence Loss of historical items 	 Pre-work inspection - Site research Clearly identify and cordon off areas of significant interest If in doubt - cease work in immediate area and cordon the site off Don't move anything Restrict access - no visitors etc Wait for site to be cleared by relevant authorities before work starts Contract Manager to okay recommencement of work Contact HBRC pollution hotline 0800 108 838

GHD

134 Queen Street East Hastings 4156 T: 64 6 870 9105

© GHD 2014

This document is and shall remain the property of GHD. The document may only be used for the purpose for which it was commissioned and in accordance with the Terms of Engagement for the commission. Unauthorised use of this document in any form whatsoever is prohibited.

N:\NZ\Palmerston North\Projects\51\31468\WP\GHD AEE Inputs\Updates May 2014\B1 - Project Description Final 28-05-14.docx

Document Status

Rev	Author	Reviewer		Approved for Issue		
No.		Name	Signature	Name	Signature	Date
0	T Dakers/M Thorne	Tony Harrison		Tony Harrison		30/09/13
1	M Thorne	Tony Harrison		Tony Harrison		23/10/13
2	M Thorne	Tony Harrison		Tony Harrison		6/12/13
3	M Thorne	Tony Harrison		Tony Harrison		28/05/14

www.ghd.com

